Implementation Process of

Pavement ME Design in Maricopa County



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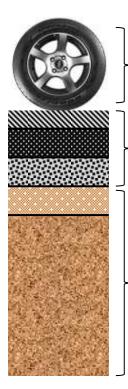
Main Objective of this Study

- Explore the new pavement design software program
- Find suitable local calibration factors for County conditions
- Implement the new design method at Maricopa County in the future





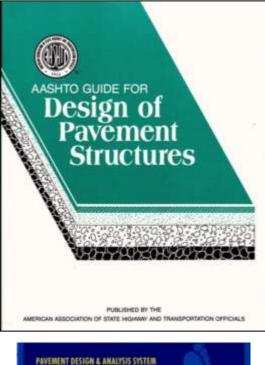
Current MCDOT Pavement Design Procedure

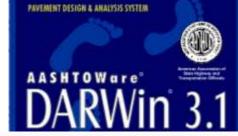


Traffic: AADT & % Trucks converted to ESALs

AC and AB: *a*_i and E

Subgrade: R-Value, Sieve and PI correlated to M_R







Current Pavement Design: Pros and Cons

Roadway Design Manual

Adopted: November 3, 1993 Updated: February 2016

distante County

2901 W. Duraugo Street

Seamly, AZ 13100



Chapter 10 **MCDOT** Pavement Design Guide

PROS

- **Inexpensive testing**
- Simple design
- layer thicknesses

Based on one AASHO Road

Test conducted in the late 1950s

The latest update was in 1993

Uses empirical relations

CONS

AASHO Road Test

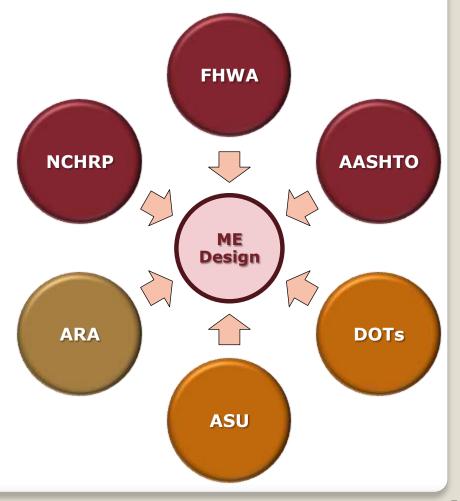




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New Pavement ME Design

- Pavement ME Design is built upon Mechanistic-Empirical Pavement Design Guide (MEPDG)
- Reflects eight years of research and development by ASU and others
- Continuous improvement under NCHRP, the FHWA and State Agencies





New Pavement ME Design: Pros and Cons

PROS

- Mechanistic behavior of structure is modeled
- Based on extensive research effort over many years
- Predicts pavement performance
- Hierarchical input levels available
- Possible to carry out local calibrations

CONS

- **Expensive testing is required**
- The design process is not very simple
- The software is expensive to maintain
- Users should gain good knowledge to input data, interpret analyses, and make reasonable decisions



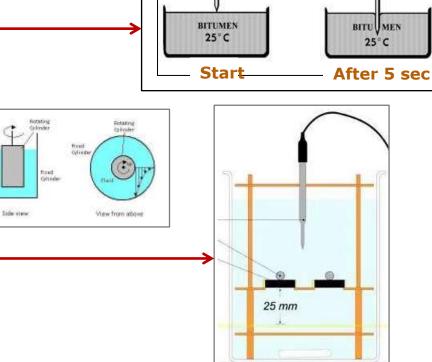
New Pavement ME Design Laboratory Testing

Binder Viscosity Testing

Penetration at 77 °F

 Brookfield Viscosity at 212, 250, 275, 300, – and 351 °F

Softening Point



100 g



Penetration

(0.1 mm units)

100 g

heater & magnetic stirrer unit

Viscosity-Temperature Susceptibility (VTS)

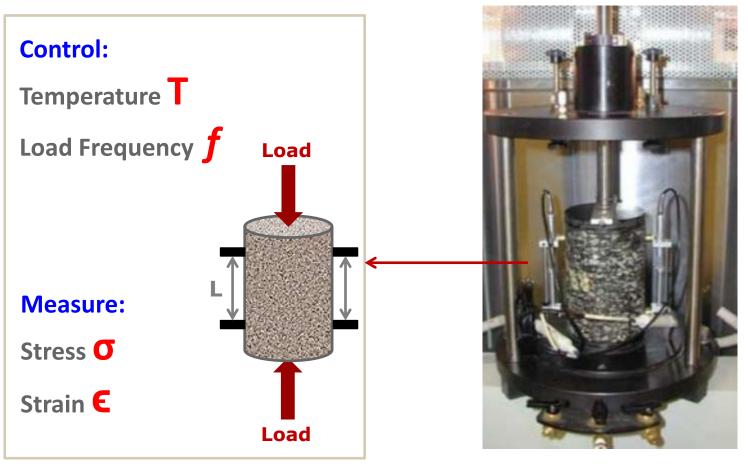


Regression parameters Ai (intercept) & VTSi (slope) describe the Viscosity-Temperature relationship

For the plot shown: Ai = 11.3383 VTSi = - 3.7955

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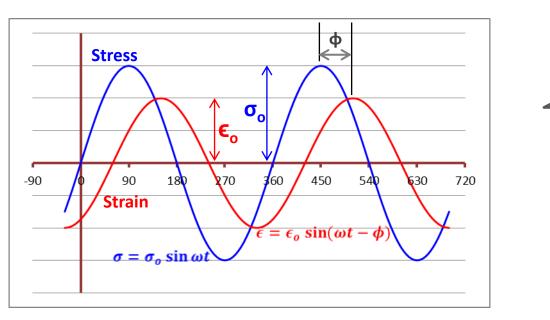
Dynamic Modulus Test

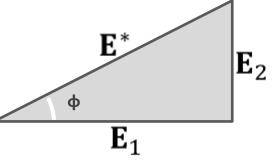


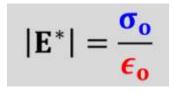


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Dynamic Modulus, E* of Visco-Elastic Material









E* in kips/in² Obtained after Testing

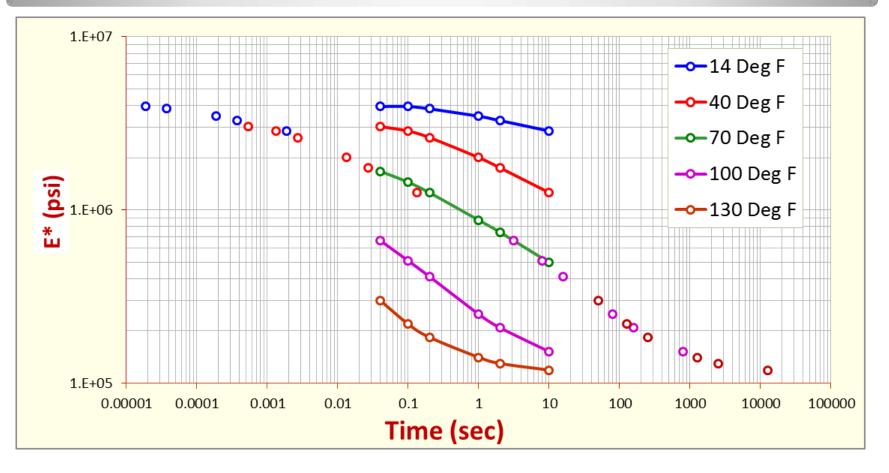
| EG | Frequency (Hz) Time (sec) | | | | | | | |
|-----------|------------------------------|-------|-------|-------|-------|-------|-----|--|
| | 0.1 | 0.5 | 1 | 5 | 10 | 25 | | |
| Temp (°F) | 10 | 2 | 1 | 0.2 | 0.1 | 0.04 | | |
| 14 | 2,854 | 3,276 | 3,470 | 3,829 | 3,947 | 3,952 | Max | |
| 40 | 1,258 | 1,749 | 2,004 | 2,614 | 2,850 | 3,030 | | |
| 70 | 497 | 743 | 872 | 1,256 | 1,445 | 1,670 | | |
| 100 | 152 | 209 | 250 | 412 | 508 | 665 | | |
| 130 | 119 | 130 | 141 | 184 | 219 | 300 | | |

Min



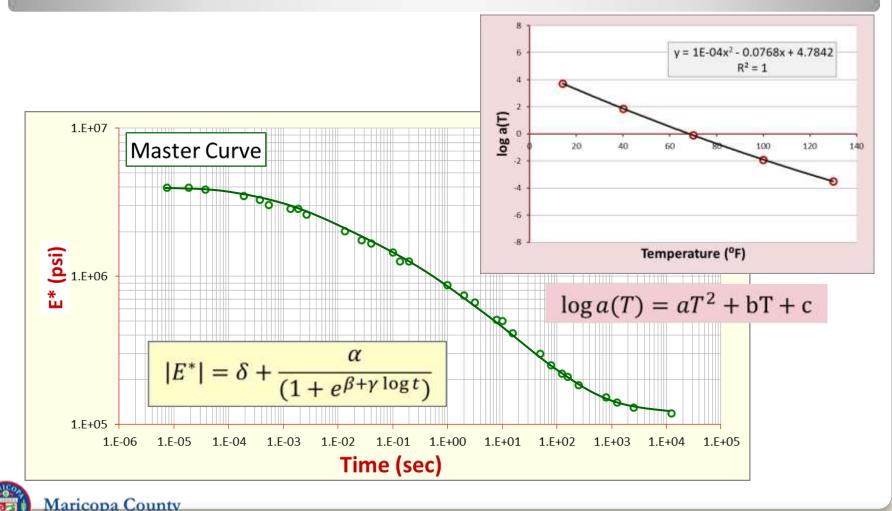
Plotting E* and Master Curve

Principal of Time-Temperature Superposition





Master Curve Function & Shift Factor Function





Maricopa County Research 2006 to 2009

- ASU conducted a research program for Maricopa County from 2006 to 2009
- All pavements were flexible pavements
- Binder, AC mix, AB, and soil samples from 15 road construction projects were collected for testing

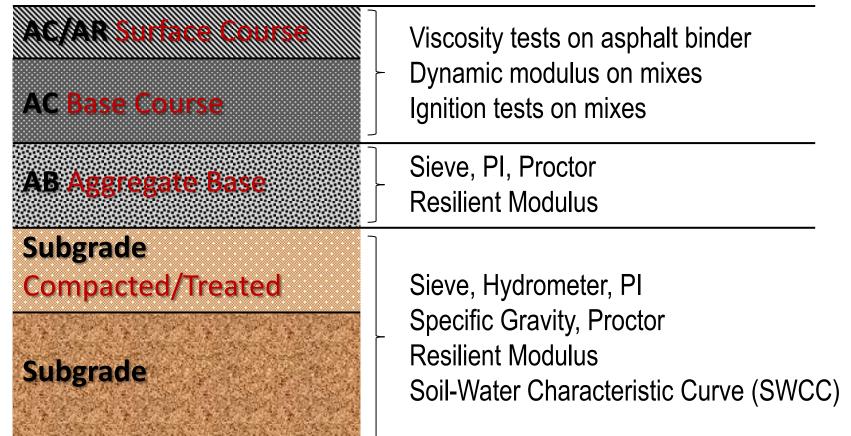






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Material Characterization under The Research Program





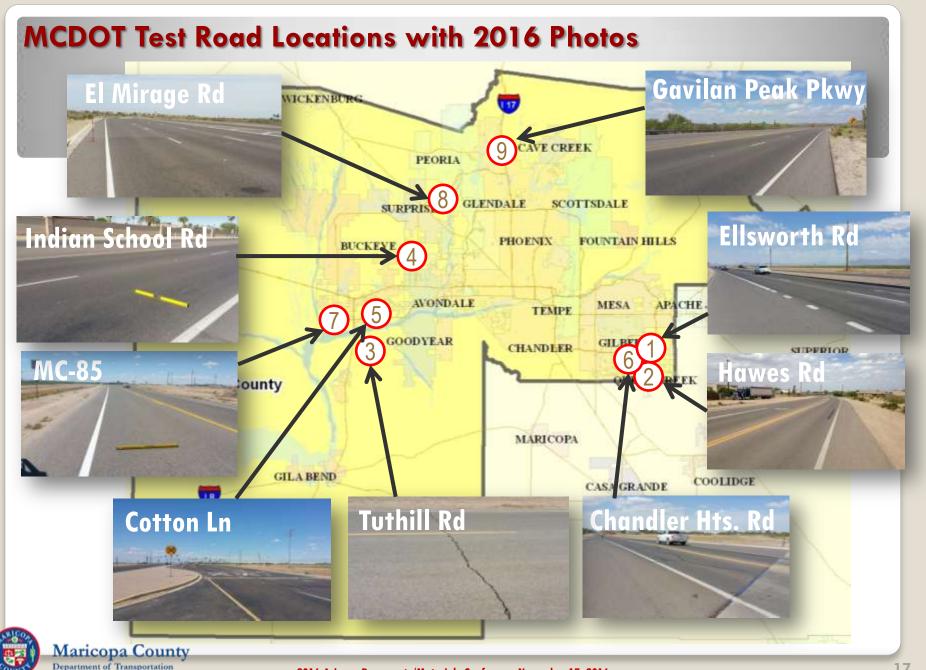
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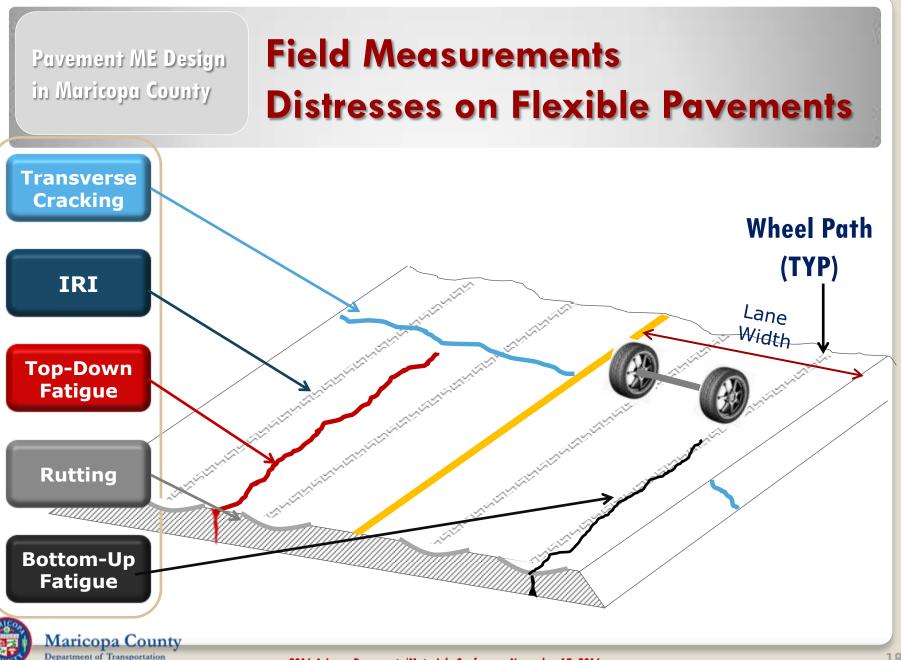
MCDOT Test Roads

Project Termini Site **Traffic On** Road No. **Project Name** ID Align. Date End Start **Ellsworth Road** EG NS Germann Rd Pecos Rd Jan-07 1 HH **Hawes Road** NS 2 Mar-07 Hunt Hwy Stacey Rd **Tuthill Road** NS 3 TQ Aug-07 **Oueen Creek Rd** Pecos Rd 10 Indian School Road Old Litchfield Rd EW **May-08** 4 **Dysart Rd** СМ **Cotton Lane** NS 5 **Cotton Ln Bridge** MC 85 Nov-07 CS **Chandler Heights** Sossaman Rd EW Mar-08 6 Hawes Rd **MC 85** 7 MT Turner Rd SR 85 EW **May-08 El Mirage Road** ED NS 8 **Deer Valley Rd** Loop 303 Mar-09 **Gavilan Peak Parkway** 9 GC NS Cloud Rd Daisy Mtn Rd **May-09**



Pavement ME Design



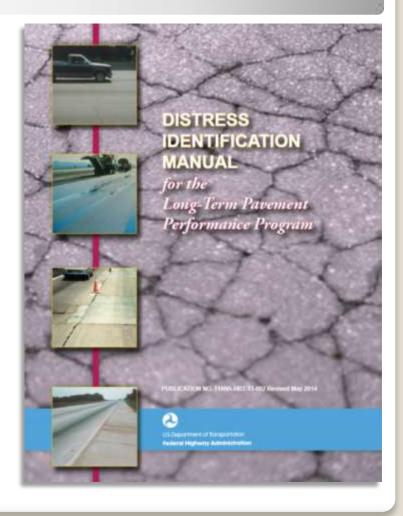


MCDOT Distress Evaluation & Database

- MCDOT distress evaluations based on LTPP Distress Identification Manual
- MCDOT Roadway Management System (RMS) Database

| De Roed 🕅 | 🖺 Include other oritaina | | | | Search For F | Roeds |
|----------------|--------------------------|-----------|--------|-------------|--------------|--------|
| Search Results | PCR, IPL SUFF Rolings | inventory | 1 | Total Miles | Lone Wiles | Selfer |
| < > 0% | Fiom | | Offici | To | | Ofset |

- IRI (International Roughness Index)
- Pavement Condition Rating (PCR)
- Sufficiency Rating
- Traffic Data
- Pavement Structure
- Work History





IRI Field Measurements

Laser Truck





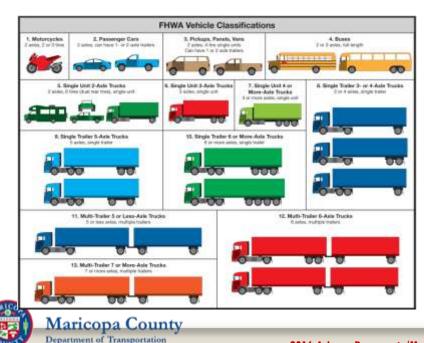


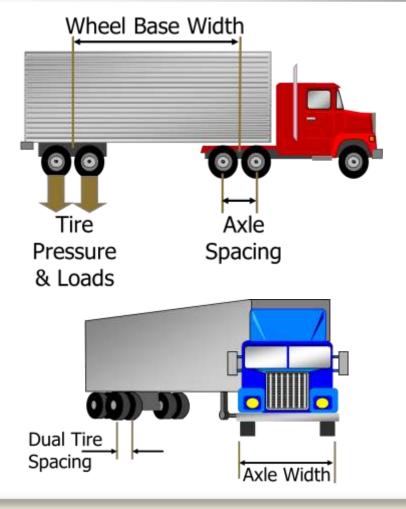


MCDOT Traffic Data & Axle Configuration Inputs

Traffic data from MCDOT Traffic Management group:

- **1.** ADT counts over the past years
- 2. Vehicle class distribution
- 3. Operational speed





Data Entry for Pavement ME Design Program



- Weather data over 20 year period
- Latitude, Longitude, and Elevation
- Depth to Groundwater Table

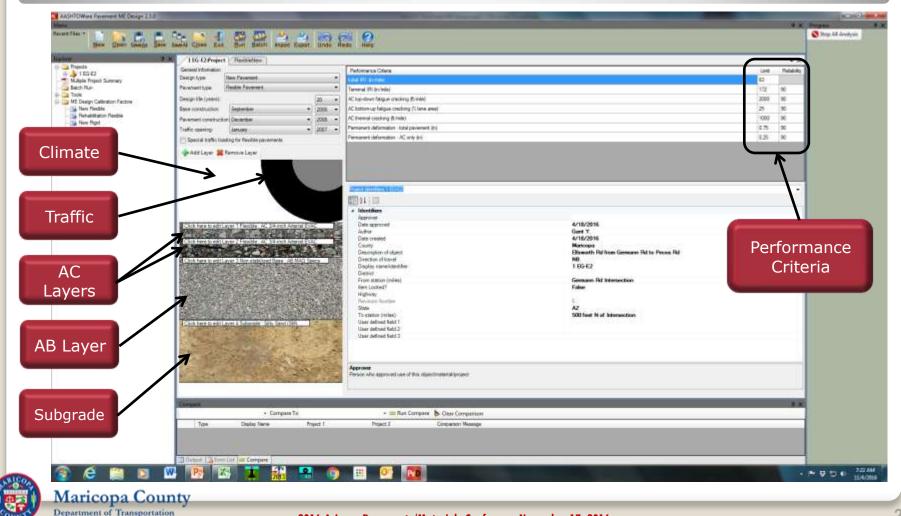


AADTT (Average Annual Daily Truck Traffic) Class Distribution (Class 4 thru 13) Axle Distribution (Single, Tandem, Tridem, & Quad)



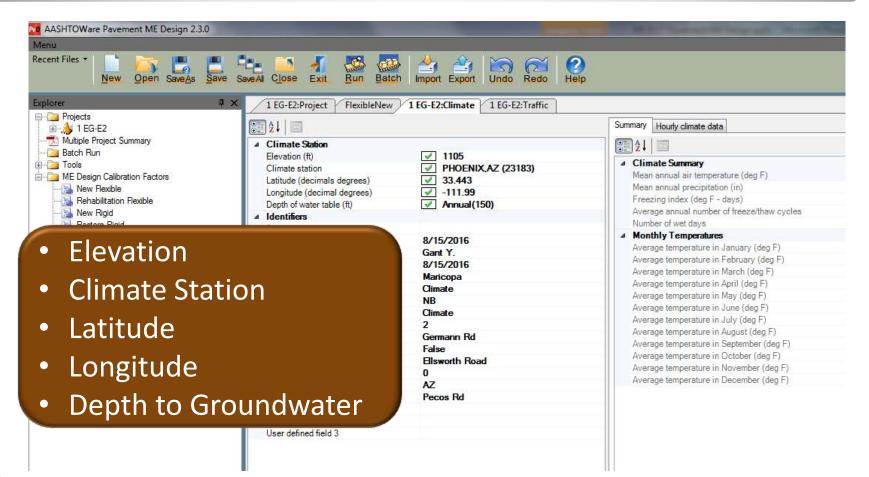
Binder Viscosity Asphalt Mix (E*, binder content, air voids) Base material (Gradation, PI) Subgrade (Gradation, PI, R-Value, Resilient Modulus, SWCC)

New Pavement ME Design Material Data



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New Pavement ME Design Climate Data





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New Pavement ME Design Traffic Data

Growth Rate (1.)

6.5

2.3

6.6

6.6

2.4

Cuise Eat Bat Satur esport Export Undo Redu Help

E2-Traffic

Vehicle Class Distribution wel Growt

Detribution (1)

7.4

25.4

13.4

42

Vehicle Class

Dans 3

Cares 6

Clean 1

Case 8 Case 9

| 1211日 | | |
|---------------------------------|-----|--------------|
| # AADTT | - | 20023 |
| Two-way AADTT | 1 | 1626 |
| Number of Janes | 1 | 4 |
| Percent trucks in design direct | 1 | 50 |
| Percent trucks in design lane | 2 | 90 |
| Operational speed (mph) | 127 | 54.5 |
| # Traffic Capacity | 100 | |
| Traffic Capacity Cap | 27 | Not enforced |
| # Axle Configuration | 100 | |
| Average asle width (ft) | 127 | 85 |
| Touris and the second second | 100 | 21.0 |

• AADTT

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- Number of Lanes
- Operational Speed
- Class Distribution
- Growth Rate
- Axle Configuration

Display Stame

Project 3

• Axles per Truck

| 19.3 | | | 6.6 | | Compound | | | lb. | |
|--------|---------|-------|-----------|--------|-----------|-----------|----------|-------------|--|
| | | | 66 | | | | | | |
| 2.4 | | | 7.5 | | Companyed | | | n and n | |
| | | _ | 66 | | Compound | | 1 1 | de de | |
| 6.5 | | | 6.6 | | Compound | 1 | 1 k. | no alla | |
| | | | | | | | 6 | Import Hand | |
| Ower 5 | Class 6 | Clair | 7 Class 8 | Care 9 | Oess 10 | Cierce 11 | Clare 12 | Clase 1 | |
| | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| ri j | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | \$.: | 1 | T. | 1 | 1 | |
| | 1 | 1 | t | 1 | 1 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | 1 | 8 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| | 1 | 1 | 1 | 1 | 1 | 1 | 1 | 1 | |
| Single | | | Tanden | | Talen | | Quet | | |
| 1.62 | | | 0.35 | | 0 | | 0 | | |
| 2 | | | 0 | | a | | D | | |
| 1.02 | | | 0.99 | | | 0 | | 0 | |
| 1 | | | 0.26 | | 0.85 | | 0 | | |
| 2.38 | | | 0.67 | | â | | 0 | | |
| 1.12 | | | 1.93 | | 0 | | 8 | | |
| | | | 1.08 | | 0.89 | | D | | |
| 1.19 | VALC: | | 0.26 | | 0.06 | | 0 | | |
| 119 | | | 14.45 | | | | | | |
| | | | L14 | | 0.06 | | 0 | | |

Growth Function

Computation

Compound

Compound

Composited

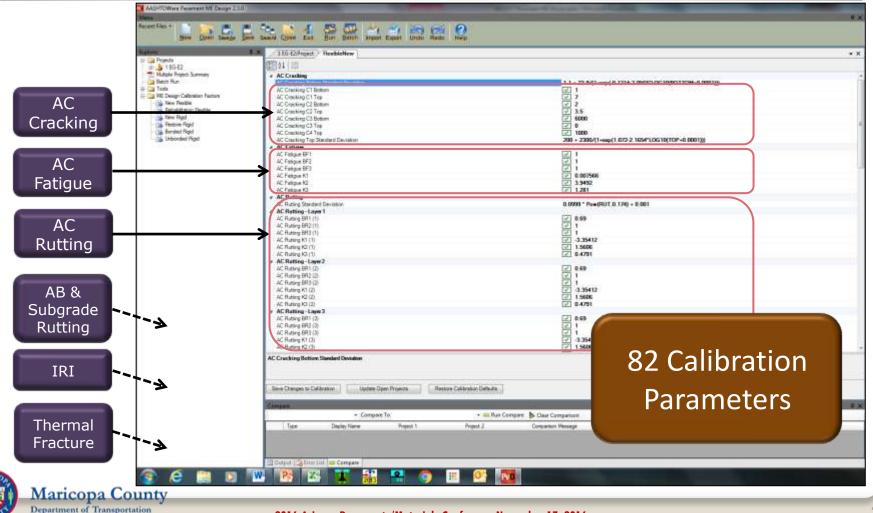
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• 3

Lond Default Distribution

New Pavement ME Design Calibration Screen



Pavement ME Design Computation Process

Integrated Climatic Model

Thermal Cracking

Asphalt Damage

Asphalt Rutting & Fatigue

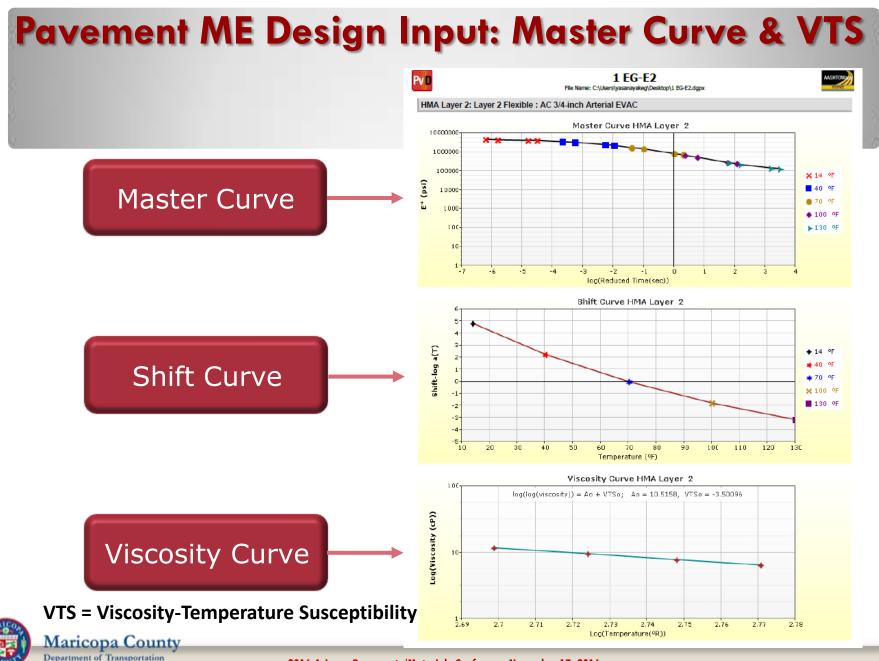
Asphalt IRI



| ts | | | | | | |
|------------------------------|---|--|--|--|--|--|
| | | | | | | |
| Design Type: FLEXIBLE Paveme | | nent construction: December, 2006 Source | | | | |
| re | | | | Traffic | | |
| Material Type | Thickness (in) | Volumetric at Con | nstruction: | And the set | Heavy Trucks | |
| AC 3/4-inch Arterial EVAC | 2.5 | Effective binder | 14.3 | Age (year) | (cumulative) | |
| AC 3/4-inch Arterial EVAC | 3.0 | the second se | 1.252.77 | 2007 (initial) | 1,626 | |
| AB MAG Specs | 10.0 | Air voids (%) | 5.8 | 2017 (10 years) | 3,623,460 | |
| Silty Sand (SM) | Semi-infinite | 1 | | 2027 (20 years) | 10,489,300 | |
| | LEXIBLE Pavem Traffic ITE AC 3/4-inch Arterial EVAC AC 3/4-inch Arterial EVAC AB MAG Specs | Material Type Thickness (in) AC 3/4-inch Arterial EVAC 2.5 AC 3/4-inch Arterial EVAC 3.0 AB MAG Specs 10.0 | Material Type Thickness (in) AC 3/4-inch Arterial EVAC 2.5 AC 3/4-inch Arterial EVAC 3.0 AB MAG Specs 10.0 | Material Type Thickness (in) Volumetric at Construction: AC 3/4-inch Arterial EVAC 2.5 AC 3/4-inch Arterial EVAC 3.0 AB MAG Specs 10.0 | Material Type Thickness (in) Volumetric at Construction: Ac 3/4-inch Arterial EVAC 2.5 AC 3/4-inch Arterial EVAC 3.0 AB MAG Specs 10.0 | |

| Distress Type | | Specified ability | Reliability (%) | | Criterion | |
|---|---------|----------------------|-----------------|----------|------------|--|
| | Target | Predicted | Target | Achieved | Satisfied? | |
| Terminal IRI (in/mile) | 172.00 | 168.82 | 90.00 | 91,54 | Pass | |
| Permanent deformation - total pavement (in) | 0.75 | 0.82 | 90.00 | 72.67 | Fail | |
| AC bottom-up fatigue cracking (% lane area) | 25.00 | 22.47 | 90.00 | 92.49 | Pass | |
| AC thermal cracking (ft/mile) | 1000.00 | 27.17 | 90.00 | 100.00 | Pass | |
| AC top-down fatigue cracking (ft/mile) | 2000.00 | 3406.60 | 90.00 | 73.15 | Fail | |
| Permanent deformation - AC only (in) | 0.25 | 0.52 | 90.00 | 3.29 | Fail | |
| | | | | | | |





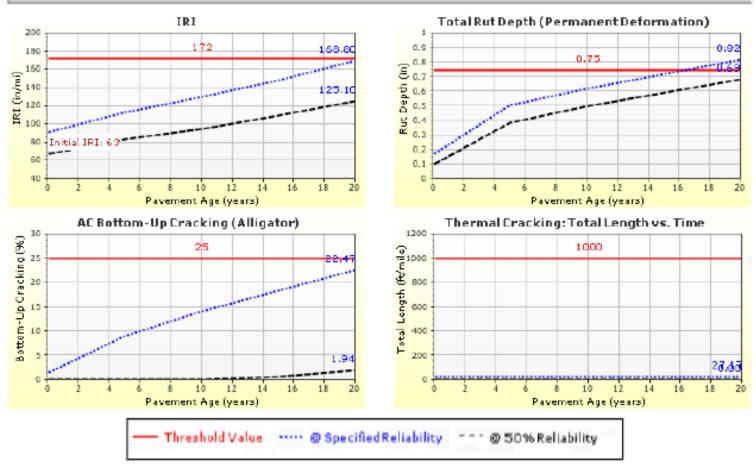
Pavement ME Design Output: Distress Charts



1 EG-E2 File Name: C:\Users\yasanayakeg\Desktop\1 EG-E2.dgpx

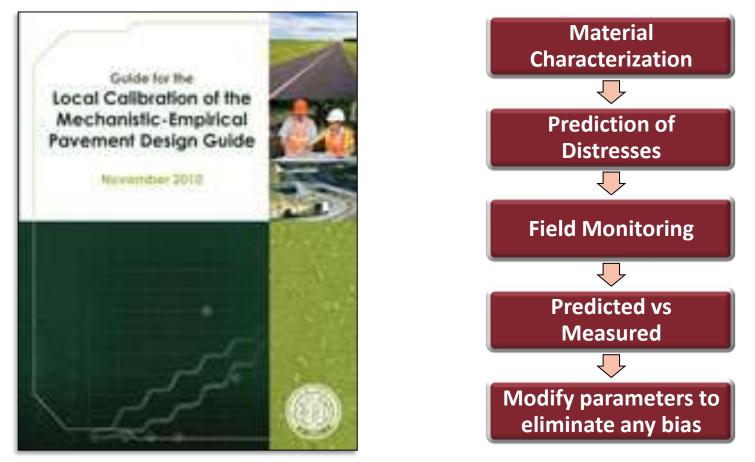


Distress Charts





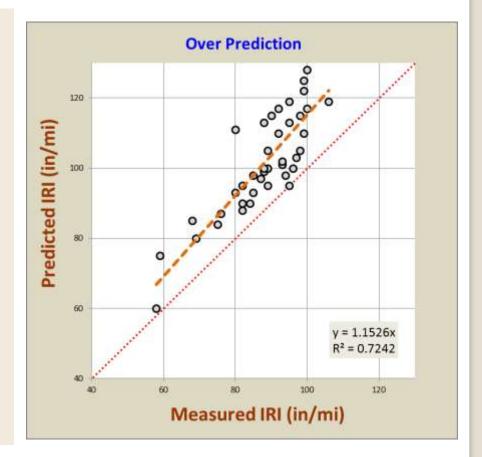
Local Calibration & Validation Procedure





Local Calibration: Bias and Goodness of Fit

- Reduce bias (avoid overdesigned and underdesigned pavements)
- Goodness-of-fit criteria is used to find the best set of calibration parameters
- Method of least squares using linear regression analysis is adopted





Approach to Local Calibration and Validation

Traditional Split-Sample

Use split-sample approach if the sample size is large

Jack-Knifing

Use jack-knife approach if the sample size is small



Model Validation Jack-Knife Method

For "n-1 Jack-Knife"

validation, remove one set of data and calibrate the model with remaining n-1 data sets

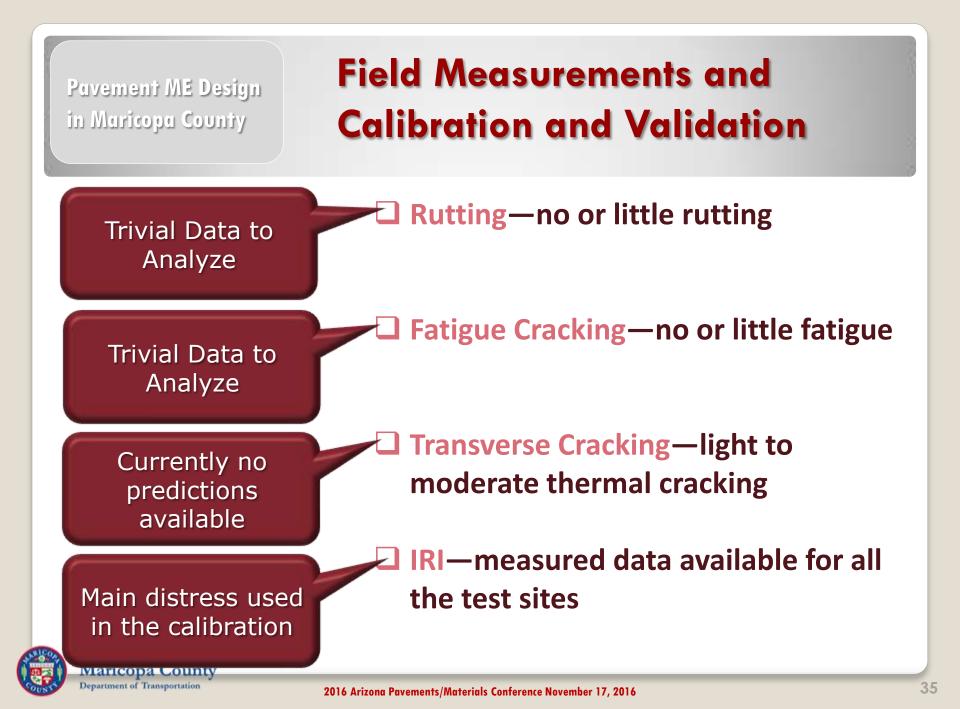
n = 9 for this study

The 9 sites were grouped into nine 1 | 8 groups as shown on the table

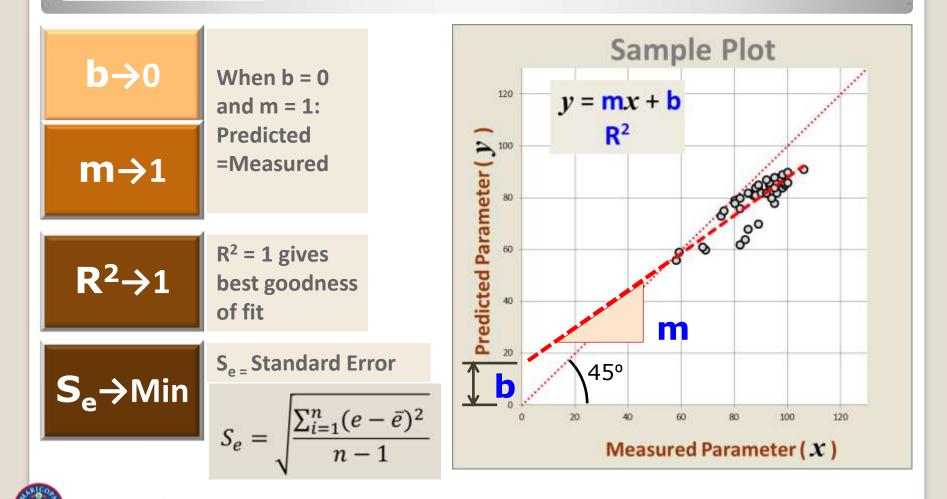
The final calibration was checked with each data set

| Site> | EG | нн | ŢQ | 10 | СМ | CS | MT | ED | GC |
|-------|----|----|----|----|----|----|----|----|----|
| 1 | С | С | С | С | С | С | С | С | V |
| 2 | С | С | С | С | С | С | С | < | С |
| 3 | С | С | С | С | С | С | < | С | С |
| 4 | С | С | С | С | С | V | С | С | С |
| 5 | С | С | С | С | < | С | С | С | С |
| 6 | С | С | С | V | С | С | С | С | С |
| 7 | С | С | V | С | С | С | С | С | С |
| 8 | С | V | С | С | С | С | С | С | С |
| 9 | V | С | С | С | С | С | С | С | С |





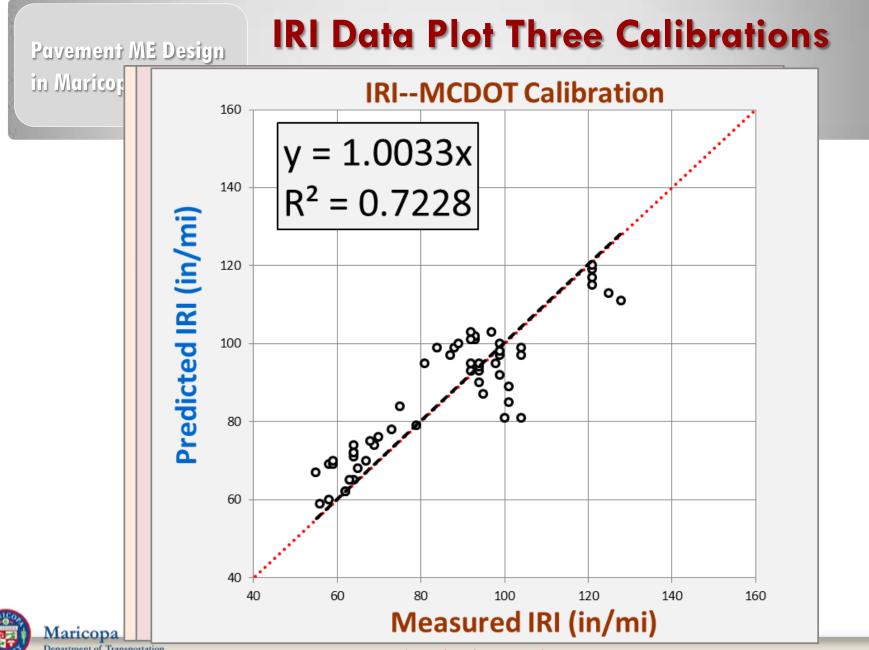
Statistical Parameters



Calibrated Parameters

| | Parameter | | | | | | | | |
|-------|-----------------------|------------|-----------------|--------------|------------------|--|--|--|--|
| | C2 | C 4 | BR1 | BS1 | BS1 | | | | |
| | Bottom Up Cracking | IRI | Asphalt Rutting | Base Rutting | Subgrade Rutting | | | | |
| NAT'L | 1.0 | 0.015 | 1.00 | 1.00 | 1.00 | | | | |
| ADOT | 4.5 | 0.028 | 0.69 | 0.14 | 0.37 | | | | |
| MCDOT | 2.0 | 0.033 | 0.69 | 1.00 | 1.00 | | | | |





Model Validation: Three Independent Sites

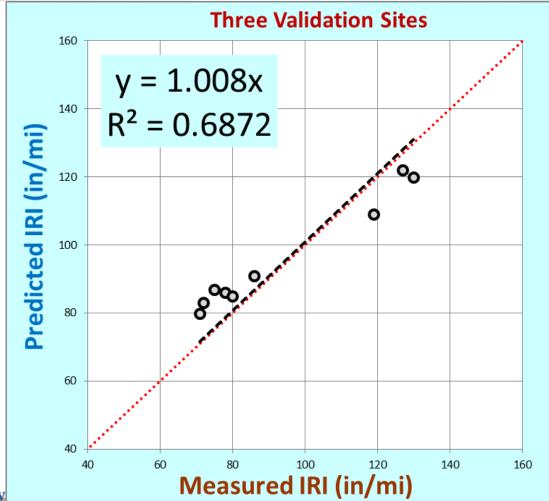
| Sites | 10 | GS | Gilbert Road | South of Salt River | North of Salt River | NS | Jul-11 |
|-------|----|----|----------------|---------------------|---------------------|----|--------|
| ier S | 11 | ST | 7th Street | Tanya Road | Desert Hills Dr | NS | May-11 |
| Other | 12 | EU | Ellsworth Road | University Dr | Adobe Road | NS | Feb-06 |
| | | | | | | | |

- Data from 3 additional roadway projects were used for validation
- These three sites were never used in the calibration process



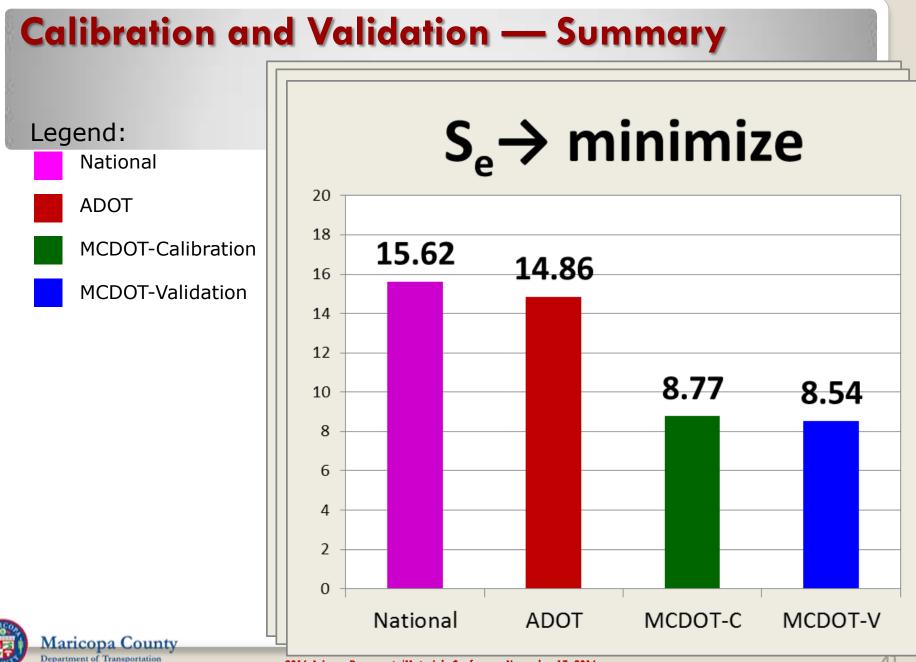


Validation: IRI Data Plot with MCDOT Calibrations





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Future Work

- Continue monitoring the test sites and the implementation process of Pavement ME Design
 - Prepare an interim MCDOT Pavement Design Guide and start designing MCDOT pavements using it
- Investigate the possibilities of modeling transverse cracking
- Provide useful suggestions to AASHTO and ARA to solve the problems encontered while using the software



Acknowledgements

- ASU
- ADOT
- ARA
- AASHTO
- Terracon
- MCDOT Traffic Group
- MCDOT Materials Lab
- MCDOT Pavement Management Group



Consulting Engineers & Scientists

Maricopa County Department of Transportation



Thank You!



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