



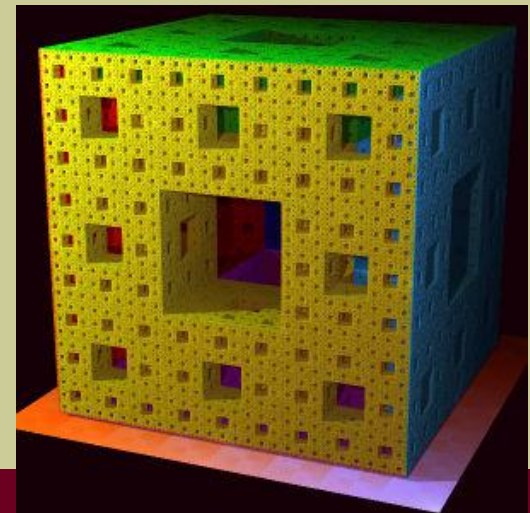
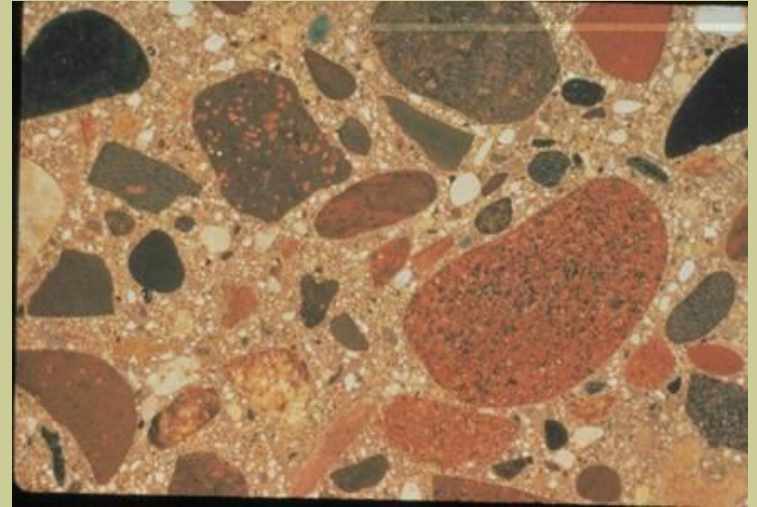
Plastic Shrinkage Cracking in Concrete

Barzin Mobasher

ASU/ADOT Pavement Materials Conference, 2011



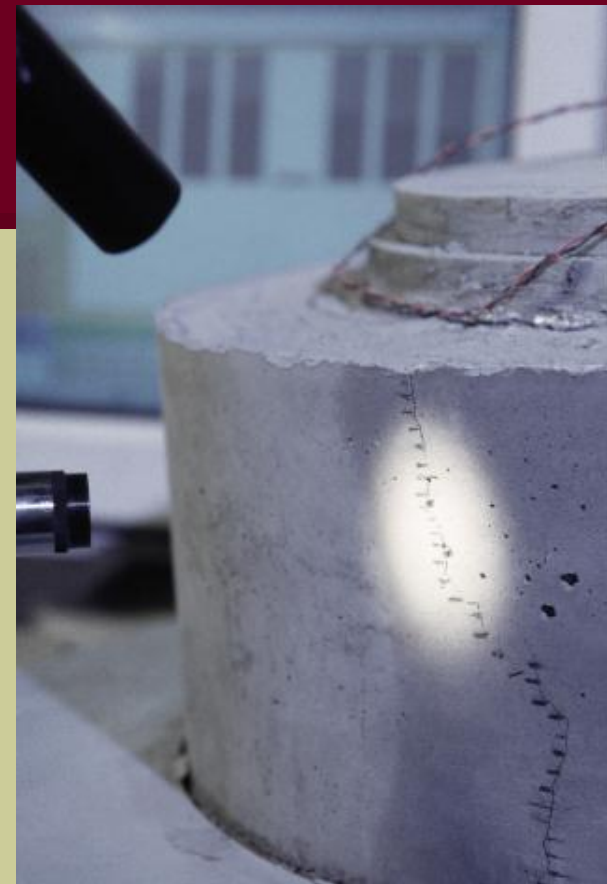
- Calcium silicate hydrate, CSH,
 - 50 -60% volume of HCP (hydrated cement paste)
- Calcium Hydroxide, CH
 - 20-25% of HCP
- Calcium sulfoaluminates
- Water
- Pores
 - Capillary pores
 - gel pores



Decrease in Concrete Volume under constant Temperature (Independent of Loading)

Parameters Affecting Shrinkage:

1. Time
2. Water Content or W/C
3. Aggregate Type
4. Ambient Condition (Humidity, Temp., Wind)
5. Curing Condition
6. Shape Effect (Volume/surface area Ratio)





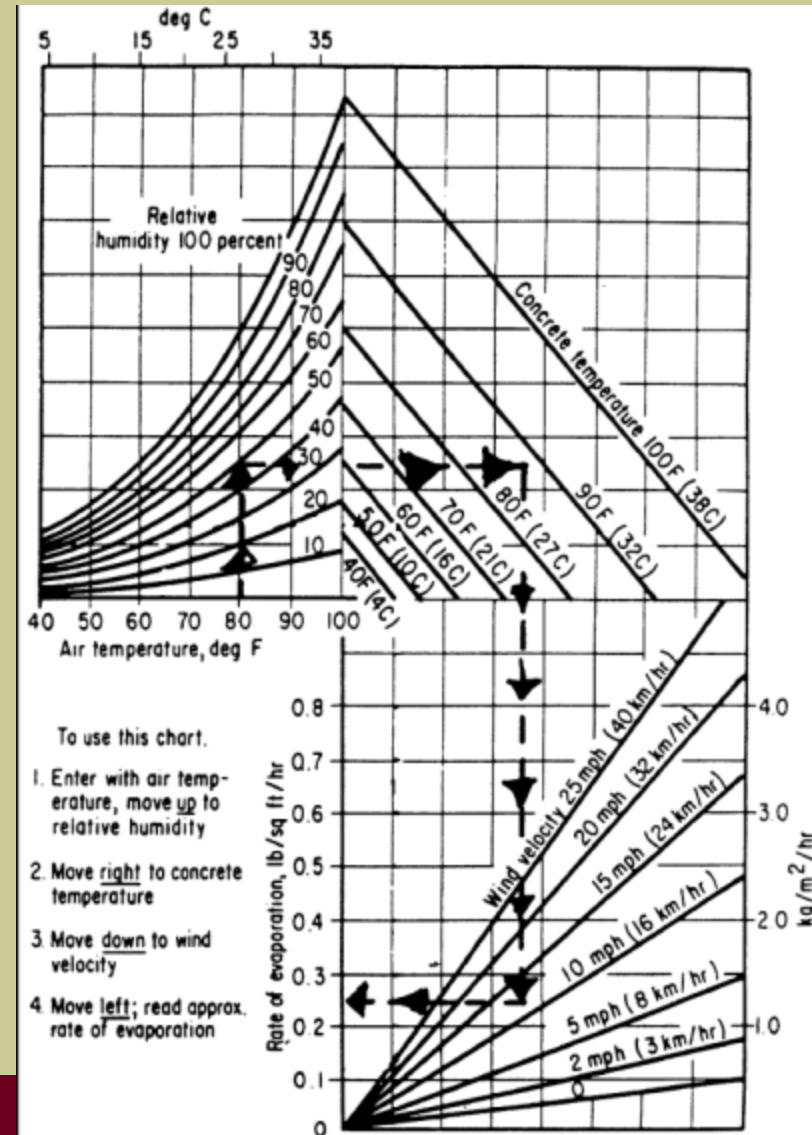


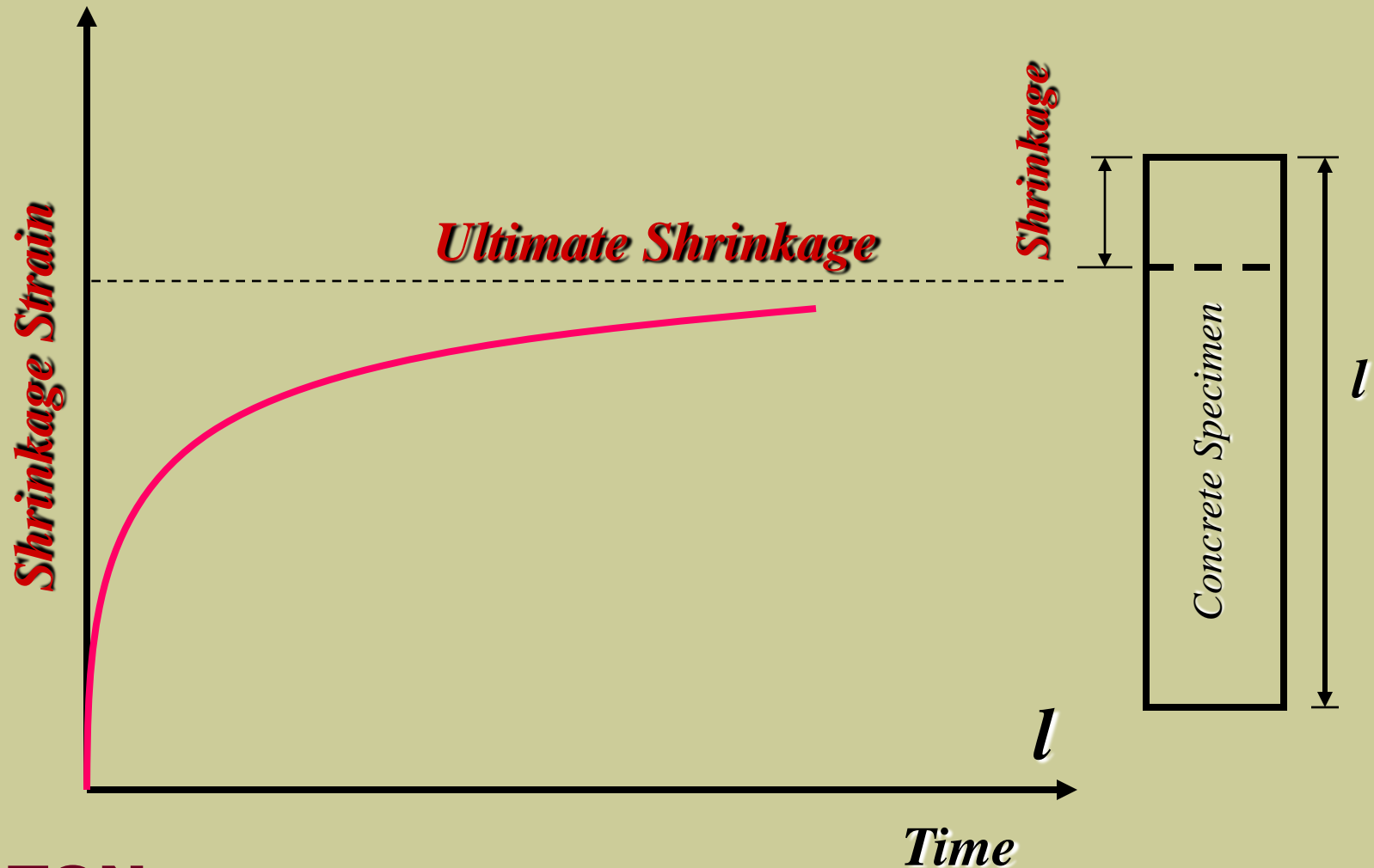
Hot Weather Concrete Standard of Practice

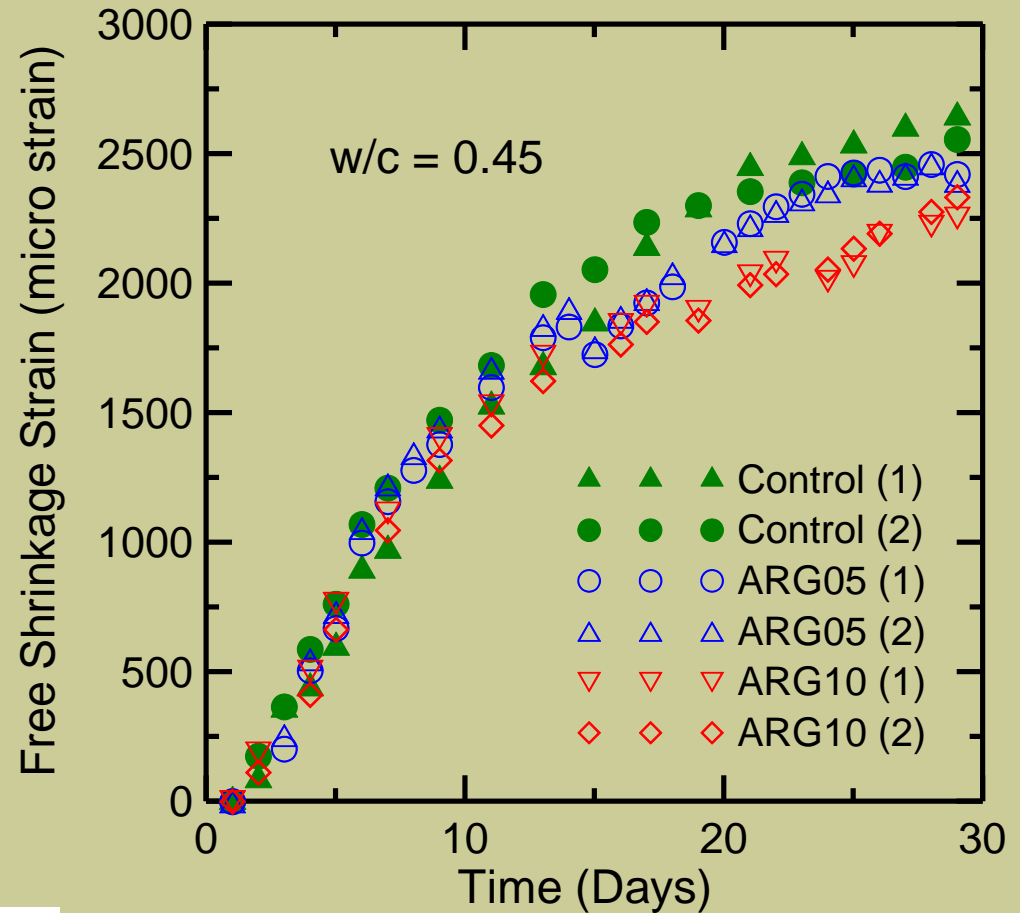
■ Origin of ACI nomograph for estimating rate of evaporation of concrete is **MENZEL'S FORMULA**



■ Based on lake Hefner near Oklahoma city (1950-2)

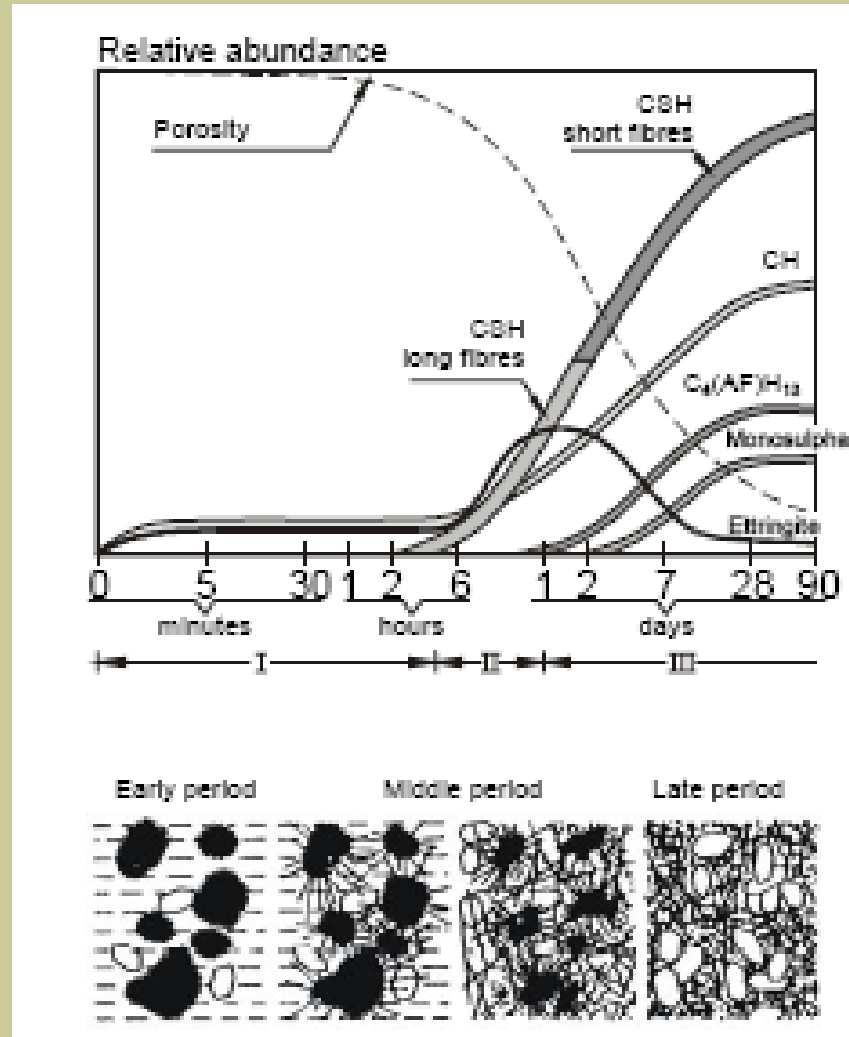




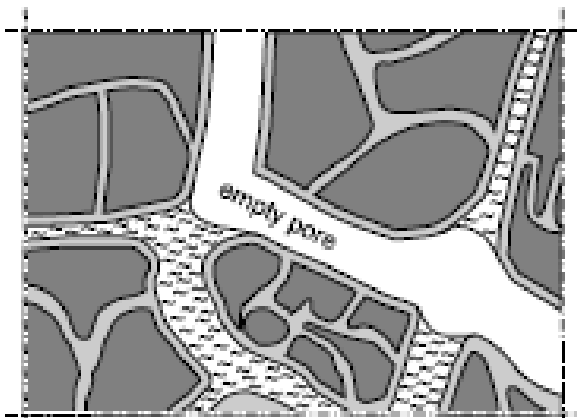
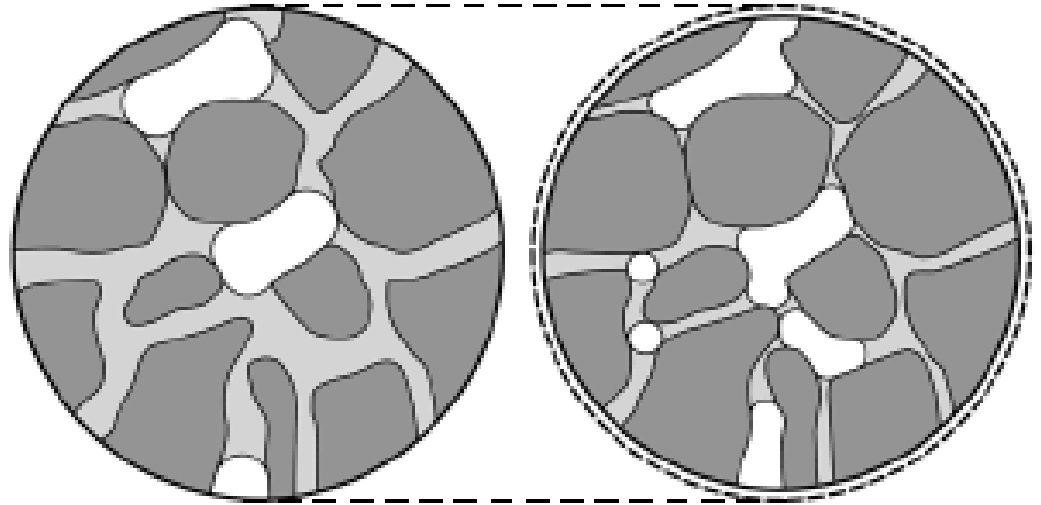


Free shrinkage parameters do not differentiate the contribution of fibers, or cracking

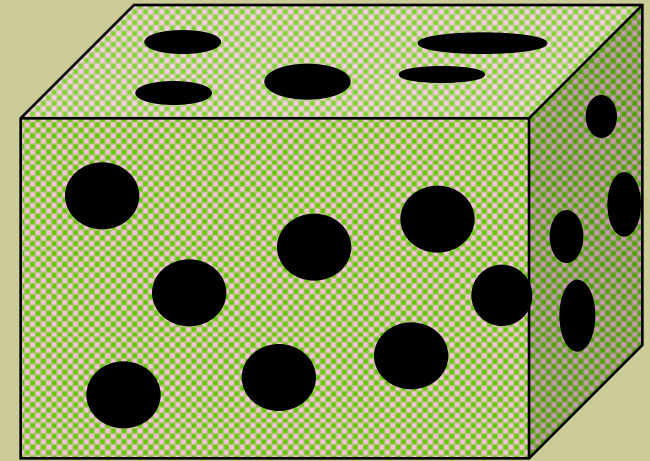
Capillary and Gel pore System after Hydration of Portland Cement



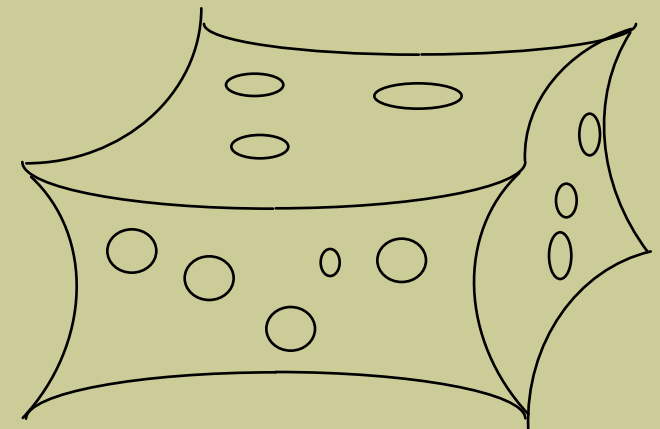
Schematics of Water removal and Internal Shrinkage



Large pores empty first, followed by smaller pores

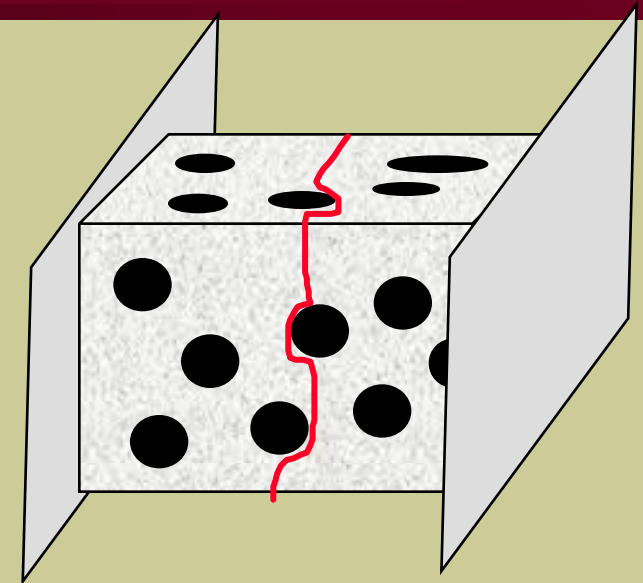
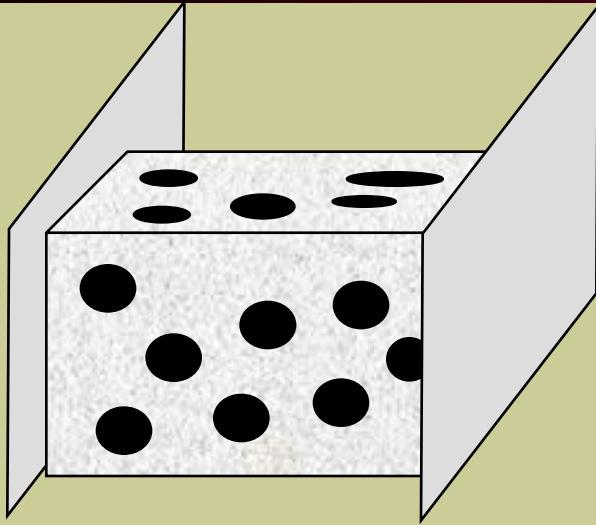


before shrinkage

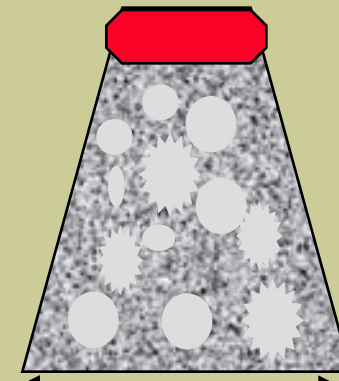
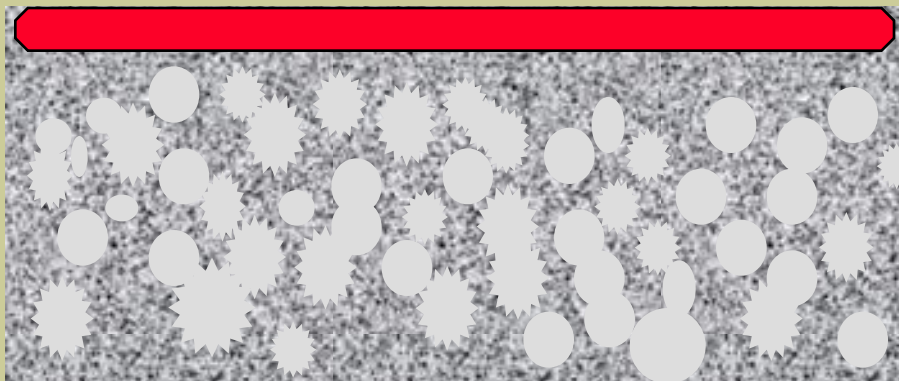


After Shrinkage

Restrained Shrinkage

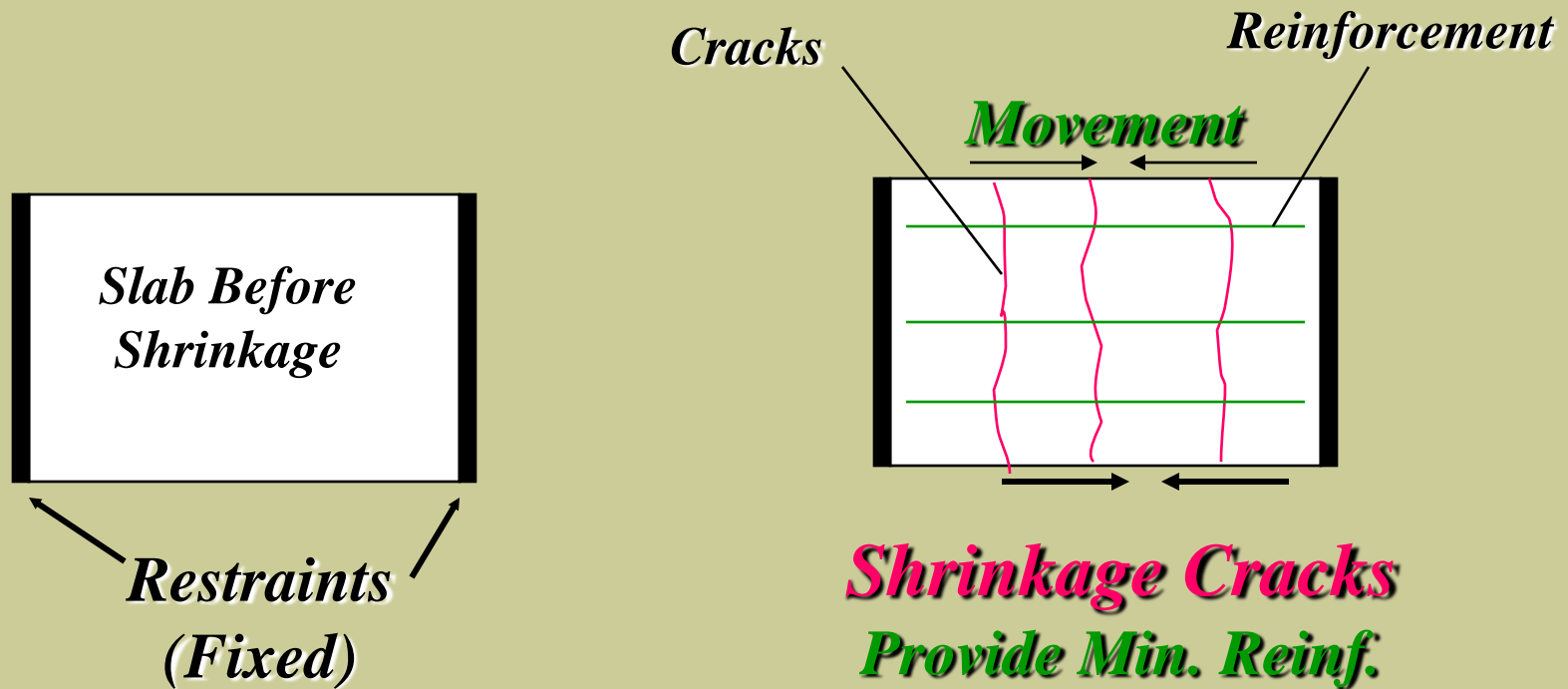


shrinkage

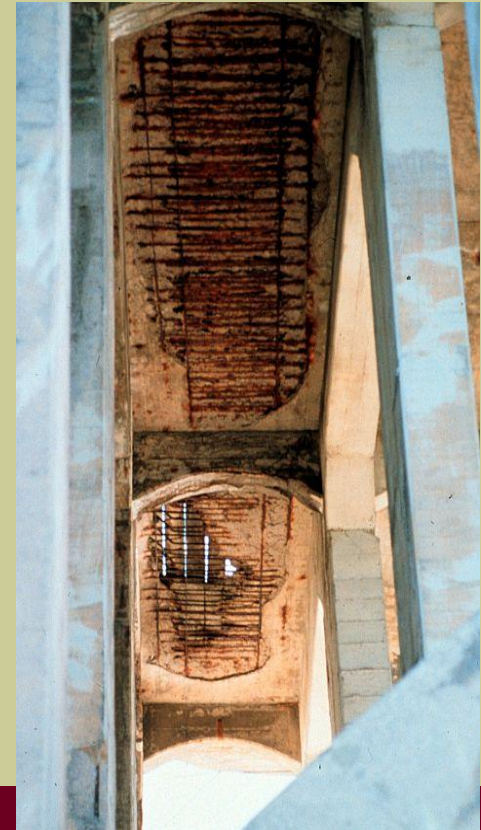
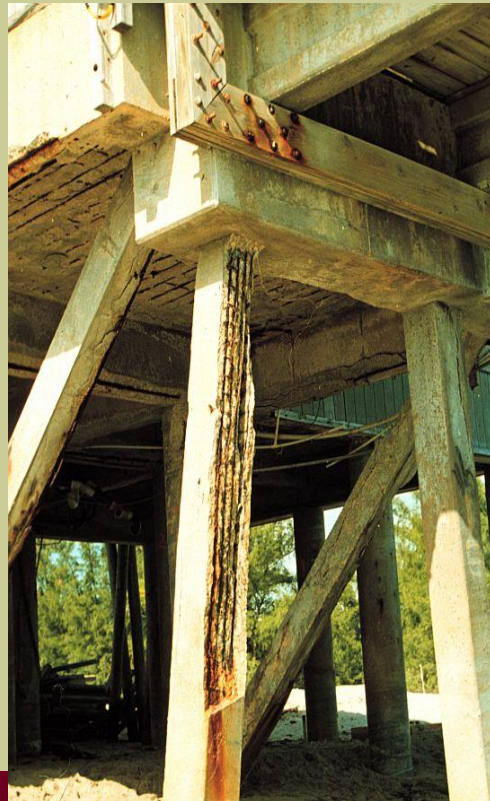


No shrinkage

- When Movement due to shrinkage can not take place freely, tensile Strain (and Stress) will occur and cause CRACKING of Concrete
- Cracking of Roof Slabs, Ground Slabs, Walls, etc



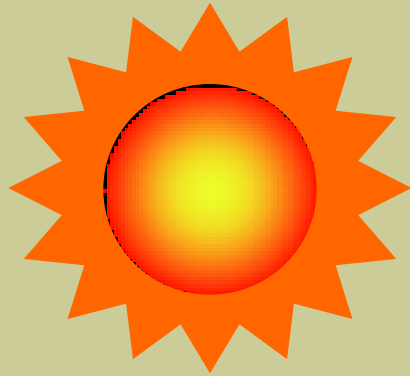
- Can high performance concrete address the problems?
- Shrinkage is a major issue due to the use of Silica fume and high paste content, with low water content.



Bleeding and its control

- problems
 - poor pumpability
 - delays in finishing
 - high w/c at the top
 - poor bond between two layers
- causes
 - lack of fines
 - too much water content
- Remedies
 - more fines
 - adjust grading
 - entrained air
 - reduce water content

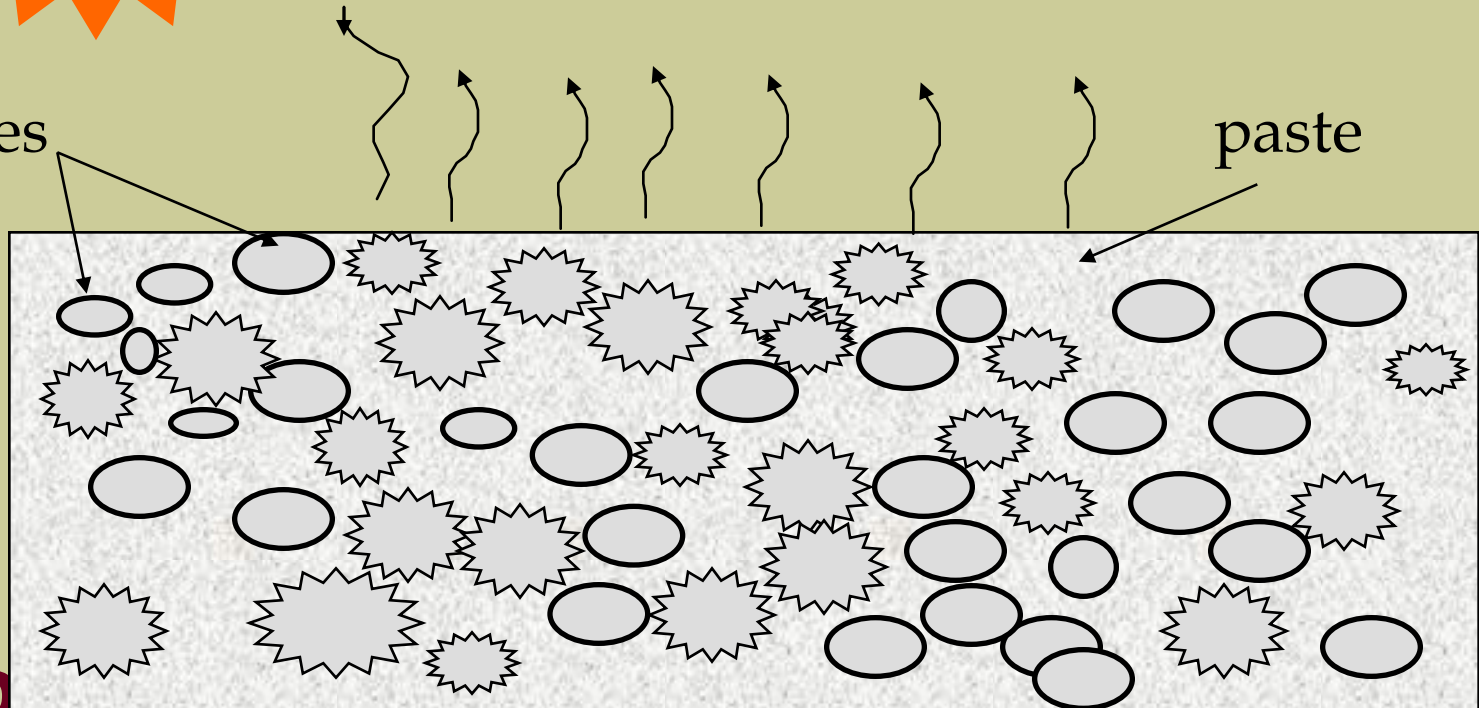
Temperature, Humidity, Wind Control

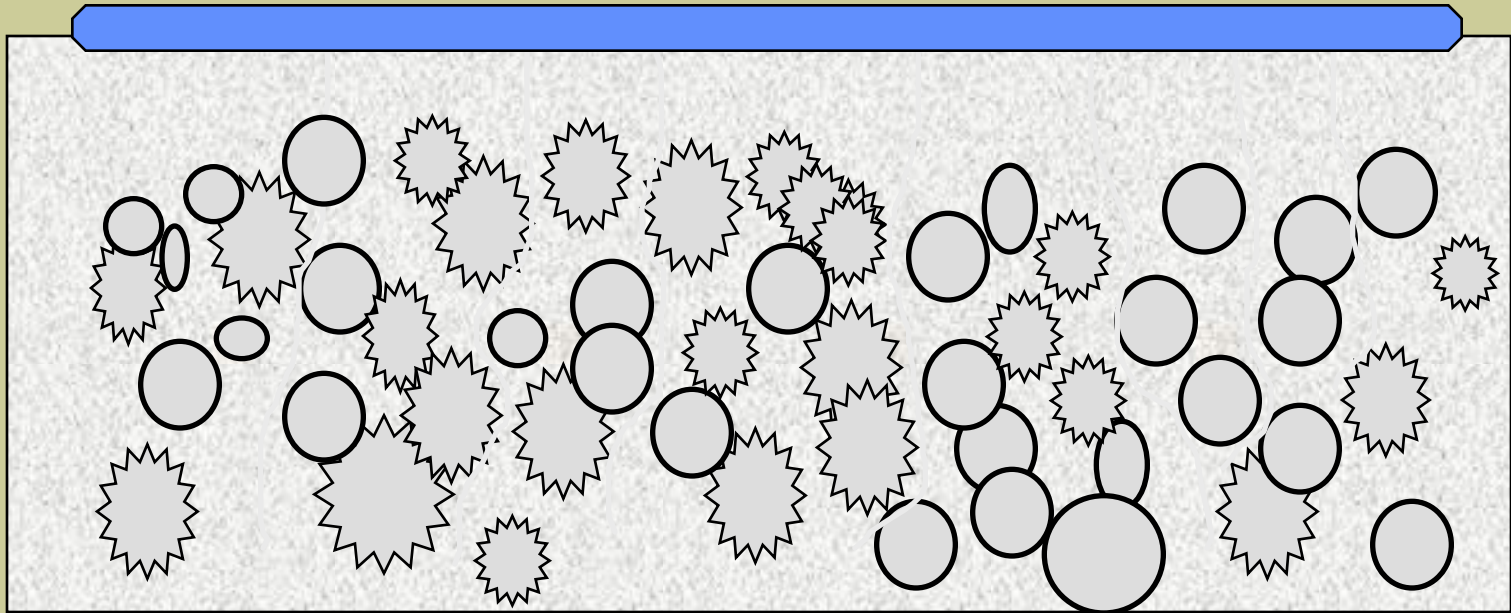


fresh concrete

paste

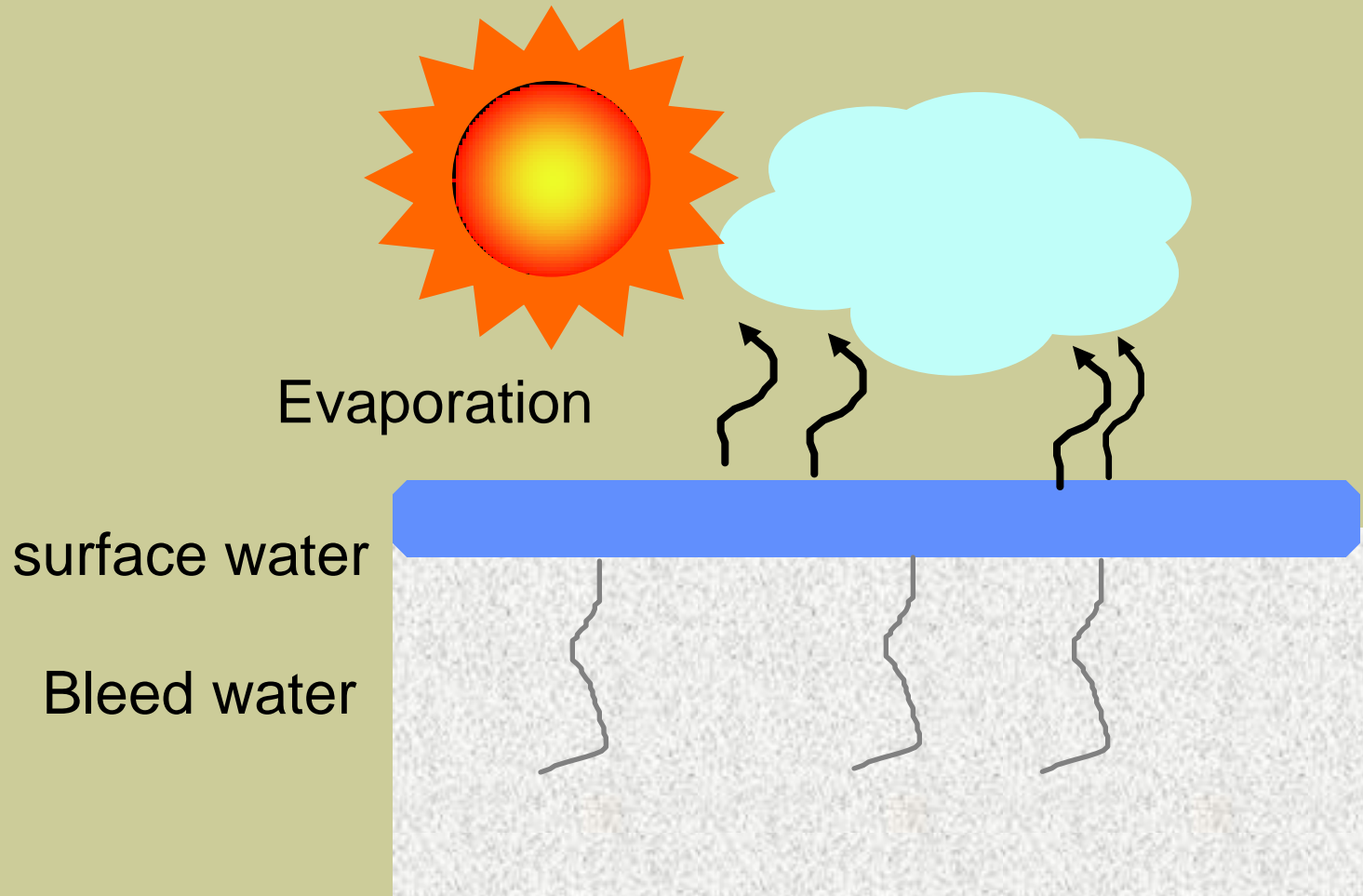
aggregates



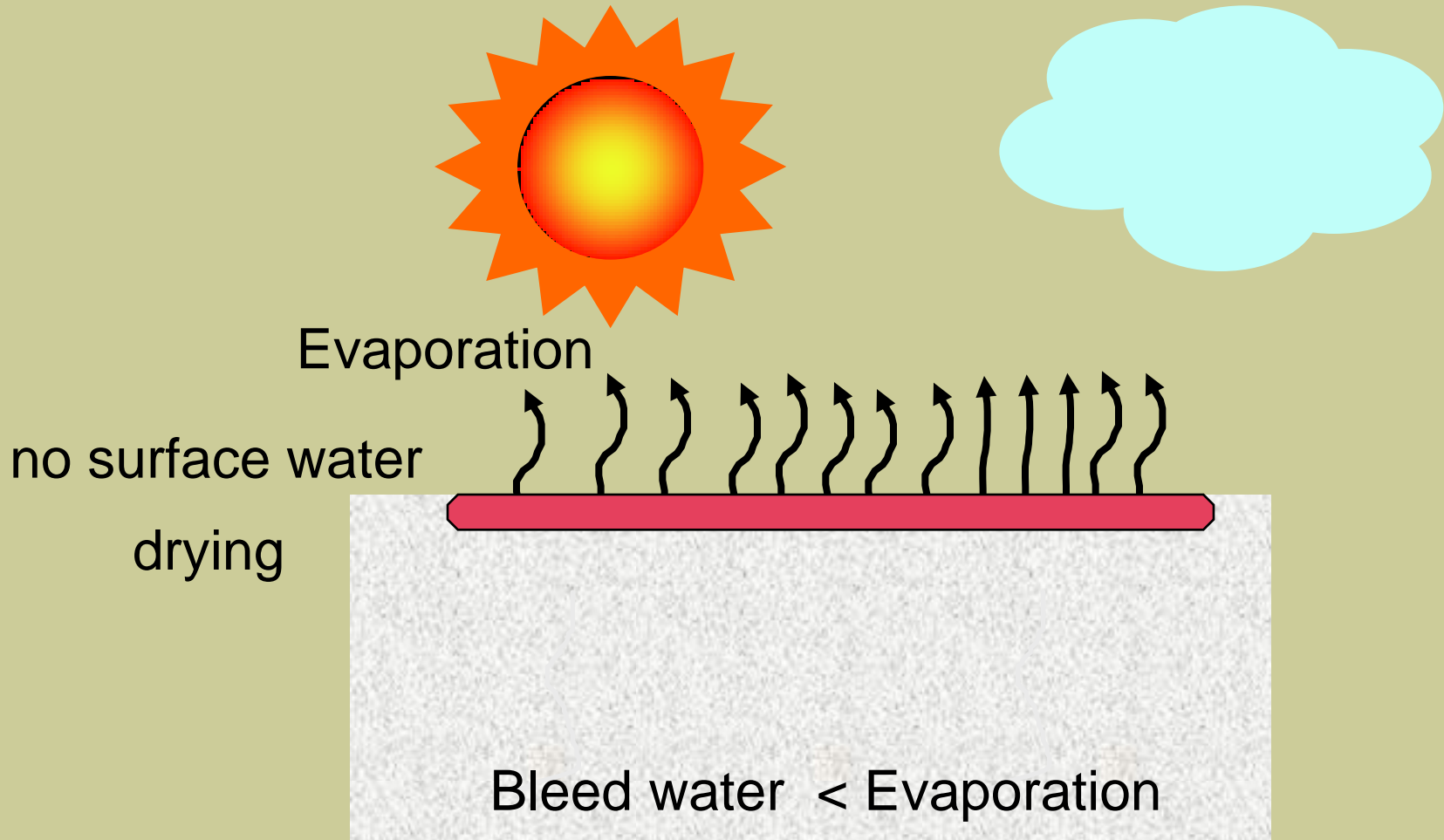




Interaction between bleeding and evaporation



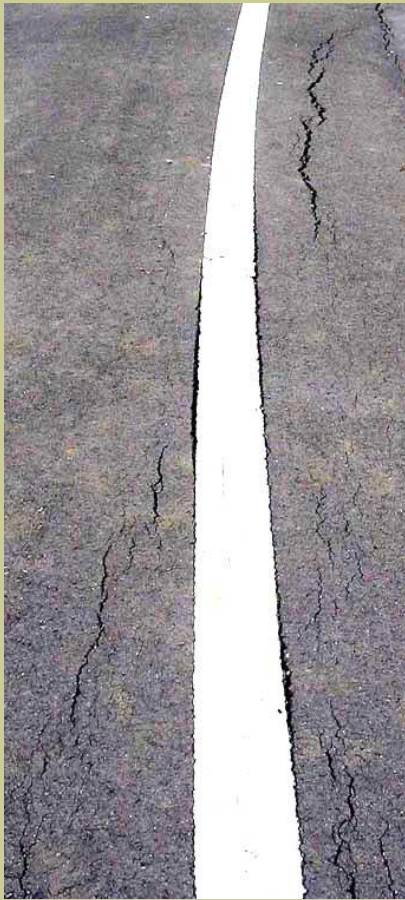
Bleed water = evaporation



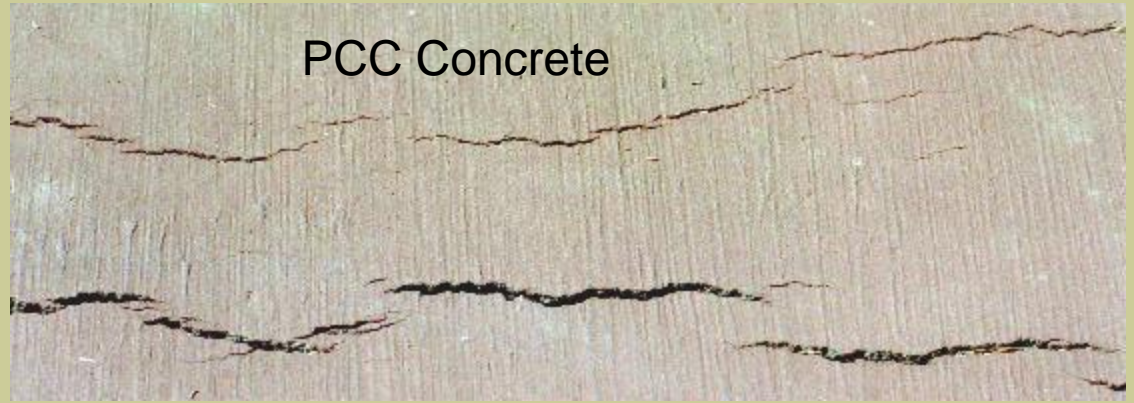


Cracks are Dominant form of failure in many engineering, art, natural events





Asphalt Concrete



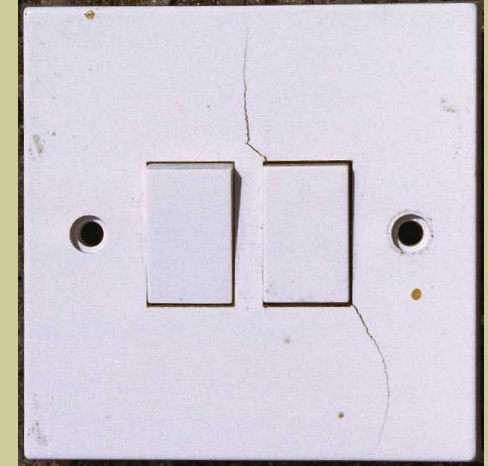
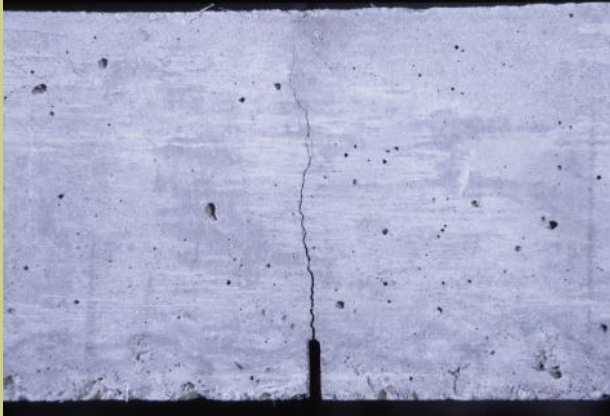
PCC Concrete



Rocks



Cracks initiate from points where stresses change rapidly



- Commonly occur in the surfaces of floors and slabs when job conditions are so dry that moisture is removed from the concrete surface faster than it is replaced by bleed water.
- Occur before the start of curing, before or after final finishing. As the moisture is removed, the surface concrete contracts, resulting in tensile stresses in the weak material.

- *Reduction of cracking tendency*
 - Lower paste Content
 - less water
 - largest practical maximum-size aggregate
 - well-graded aggregate
 - stiffer consistency
 - lower initial temperature of the concrete
 - Curing
- *Construction*
 - *Reinforcement*
 - Construction Joints

■ *Mix design*

- avoiding oversanded mixtures, using the largest maximum aggregate size practical, and using aggregate with the most favorable shape and grading conducive to workability.

■ *Effect of water content*

- Surface drying will occur except when the surface is submerged or below grade. Drying shrinkage strains of up to 600×10^{-6} or more are likely.
- Keep the concrete wet as long as possible so that the concrete will have time to develop more strength to resist cracking forces.

■ Minimum-Reinforcement

- Between 0.18 and 0.20%, does not normally control cracks to within generally acceptable design limits.
- To control cracks to a more acceptable level, min requirement needs to exceed about 0.60%.

■ Joints

- The use of joints is the an effective method of preventing the formation of cracking.
- Contraction joints in walls are made, for example, by fastening wood or rubber strips to the form, which leave narrow vertical grooves in the concrete on both faces of the wall.

Good fogging



Evaporation retarders work well but are frequently misused



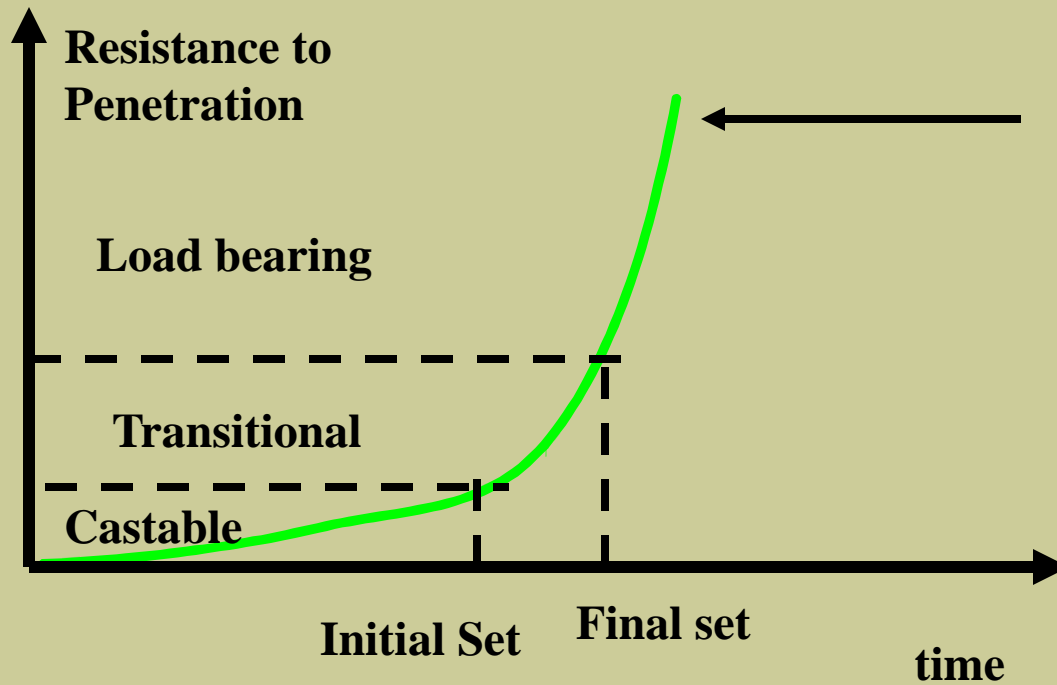
Applying curing compound while concrete is still fresh



“It is recommended that a minimum of 7 days continuous moist curing be specified.”

*Moist curing after
concrete hardens*





- Marketed by numerous chemical admixture companies for reducing drying shrinkage (and its associated cracking) in concrete.
- Most SRAs function by reducing the surface tension of the pore solution in the concrete.
- In addition to reducing or slowing drying shrinkage, this reduction in surface tension can also potentially reduce autogenous shrinkage in low water-binder ratio mortars and concretes and evaporative water loss during early age curing.

- During Hardening
 - Construction movement,
 - forms movement
 - Subgrade movement
 - Plastic
 - Plastic shrinkage
 - Plastic settlement
 - Frost Damage
 - Premature freezing
 - Scaling, crazing
- After Hardening
 - Volume instability
 - Drying shrinkage
 - Thermal change
 - creep
- Structural design
 - Design load/overload
 - Design subgrade
 - Fatigue
- Physio-Chemical
 - AAR/ASR/DEF
 - Steel Corrosion
 - Freeze thaw Cycling

Cracks in Concrete Can transfer forces if they are maintained at a very small opening

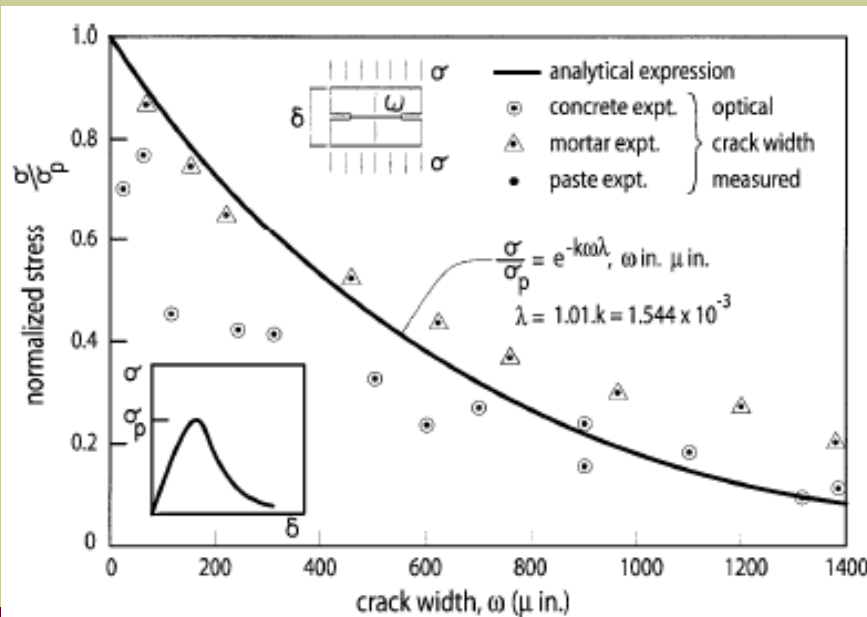
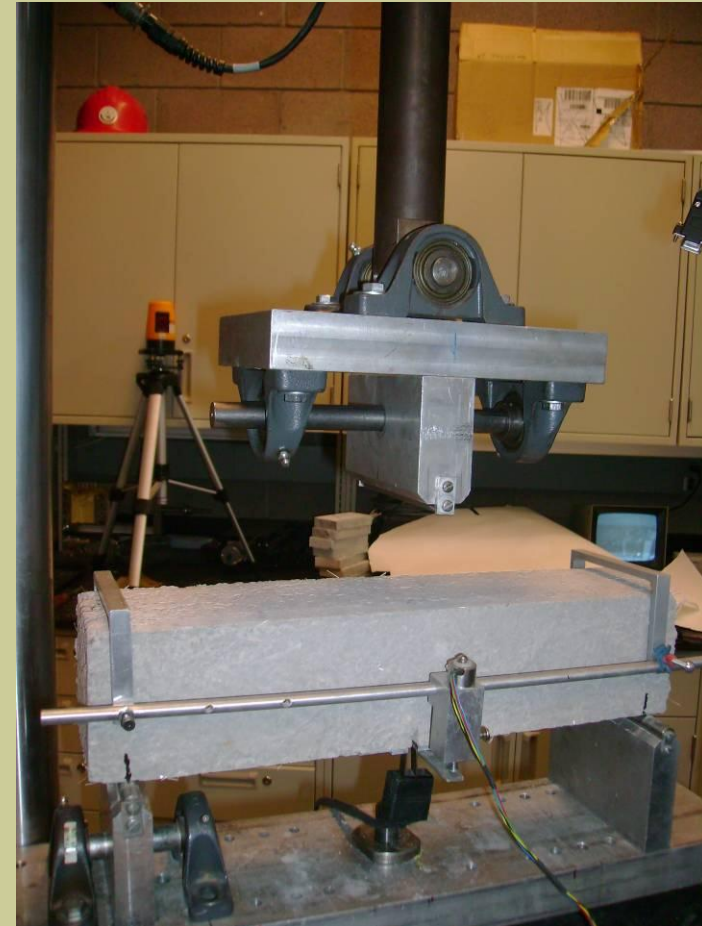


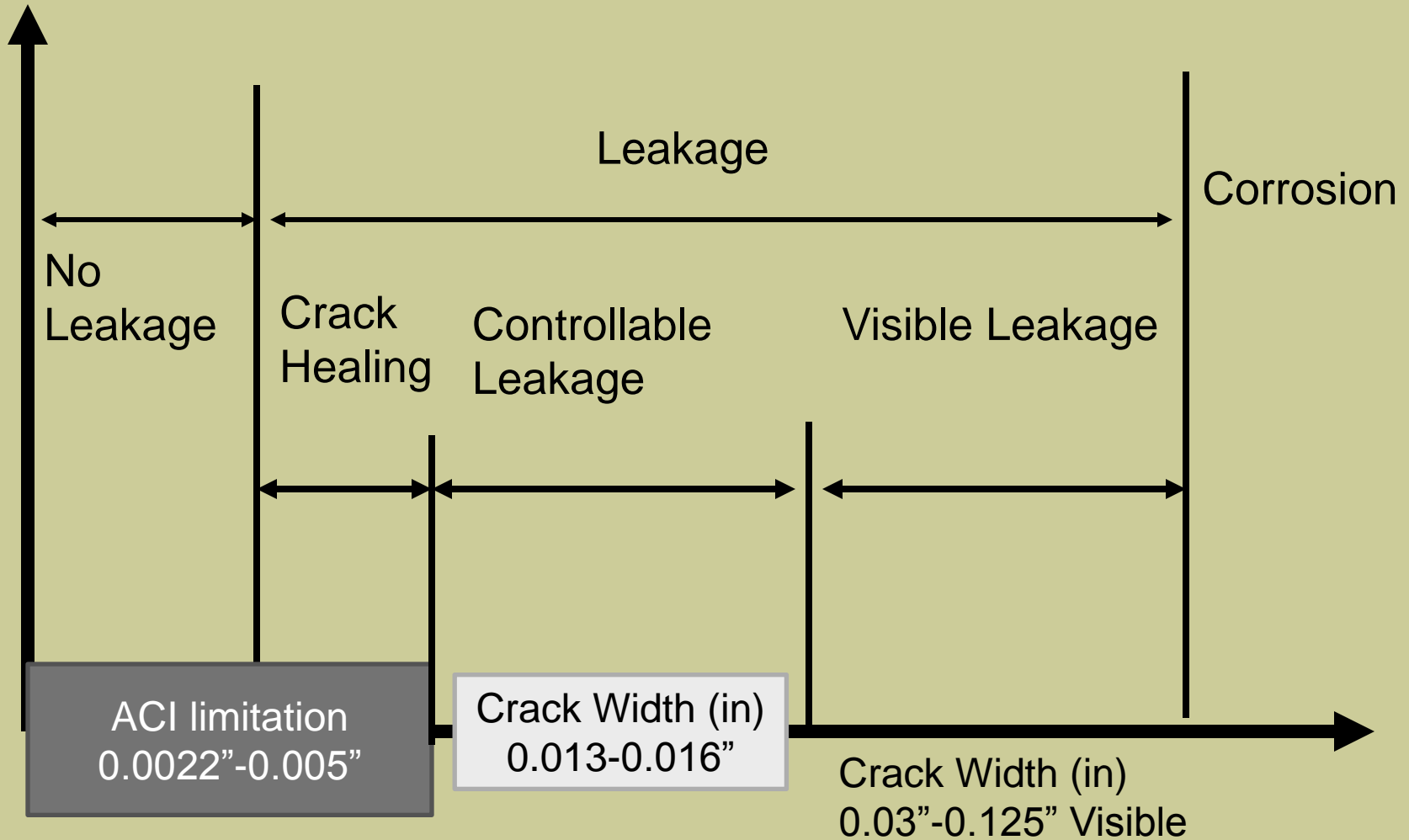
Fig. 2.8—Normalized peak stress versus crack width in uniaxial tension (Gopalratnam and Shah 1986).

exposure condition	crack width	
	in	mm
Dry Air or protective membranes	0.016	0.41
Humidity moist air, soil	0.012	0.3
Deicing chemicals	0.007	0.18
Seawater and seawater spray, wetting, drying	0.006	0.15
Water retaining structures	0.004	0.1





The Importance of Crack Width

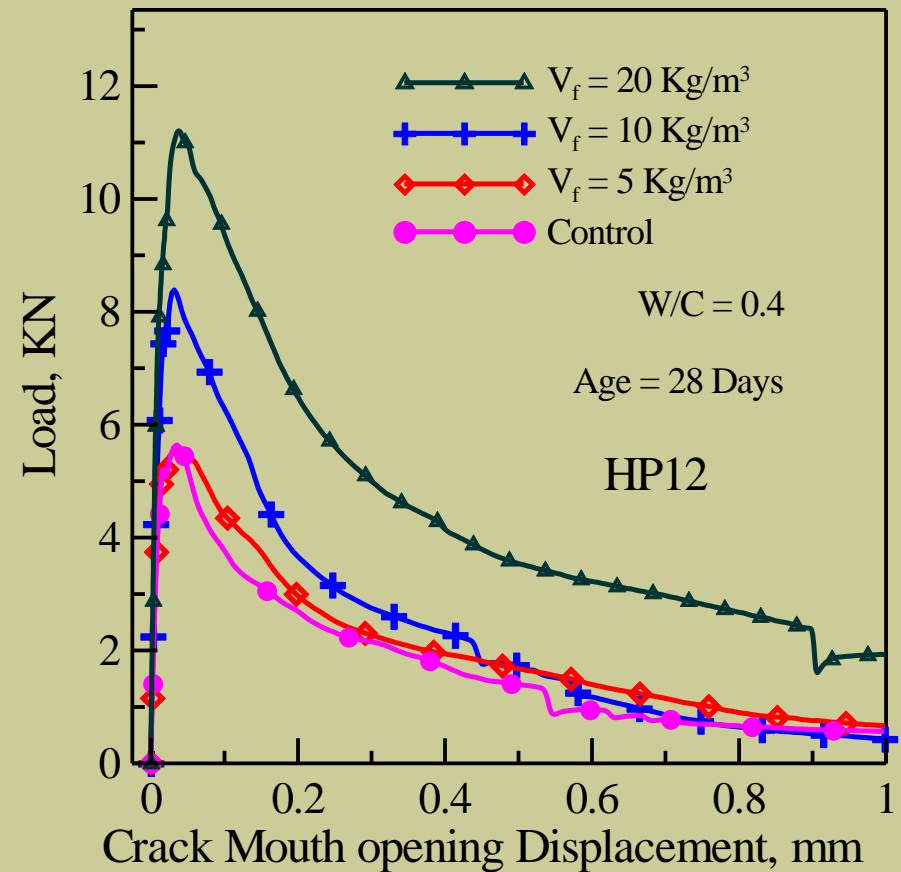
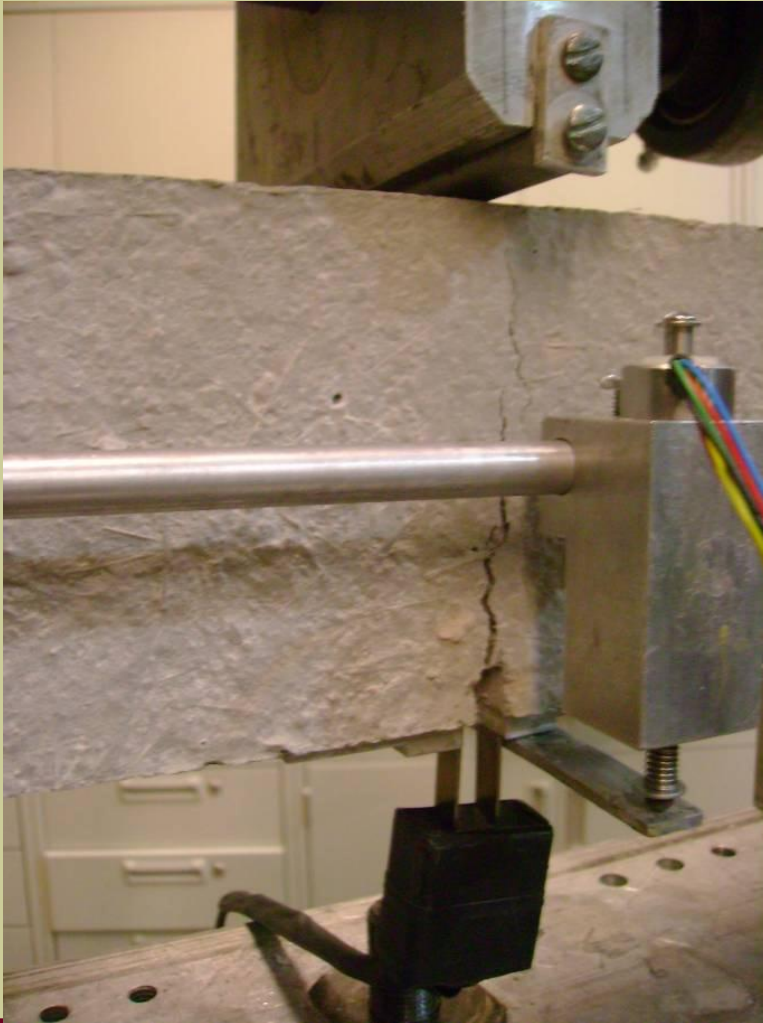


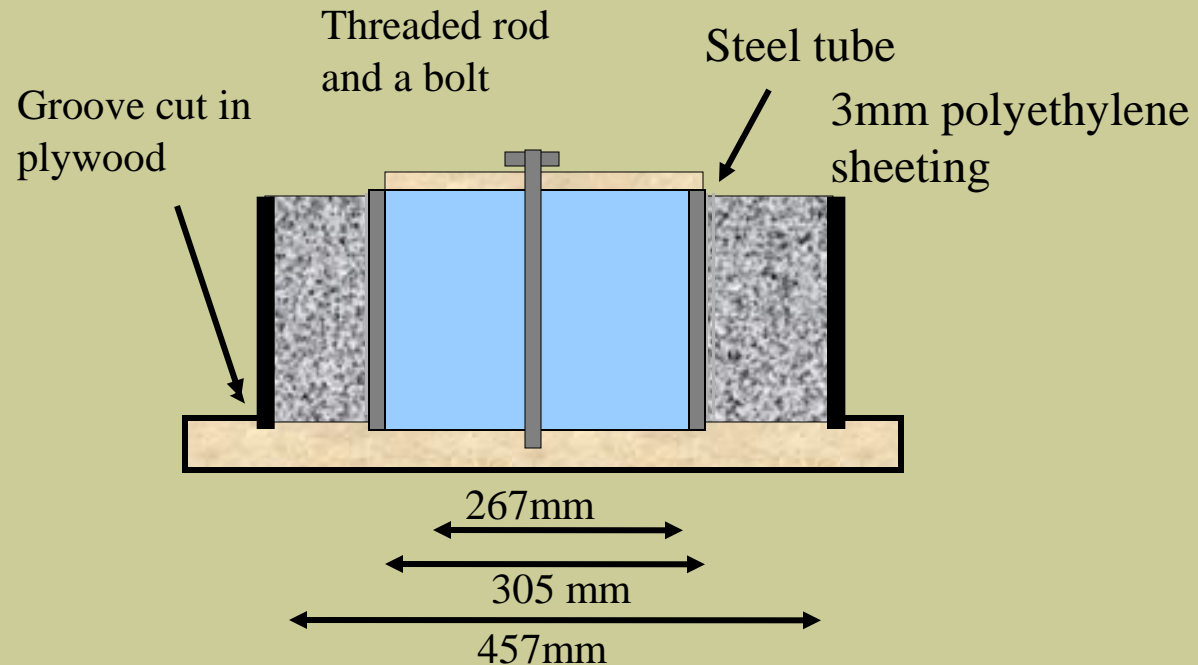
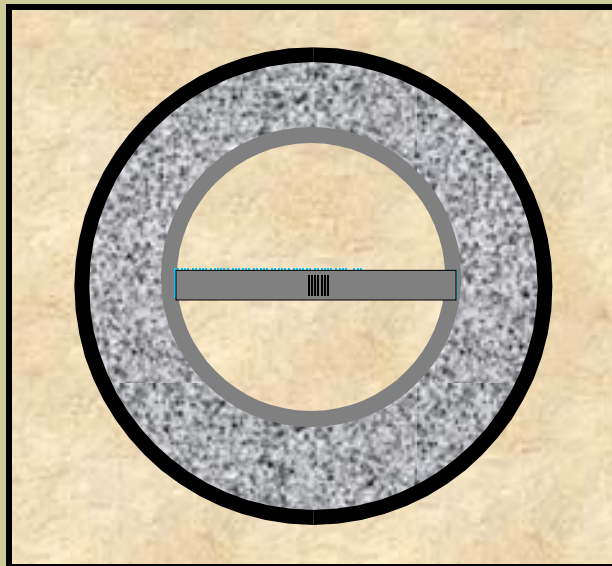


Use of Alkali Resistant Glass Fiber Reinforcement



St. Gobain Alkali resistant
Glass Fibers





Grzybowski, M.; and Shah, S. P., "A Model to Predict Cracking in Fiber Reinforced Concrete Due to Restrained Shrinkage," ACI MATER J 87 (2):138-148 Mar-Apr 1990.



restrained shrinkage specimens

FULTON

school of engineering

Constant humidity chamber subjected to constant flow of air around the specimens.



Crack width Measurements using a microscope with a 10x objective



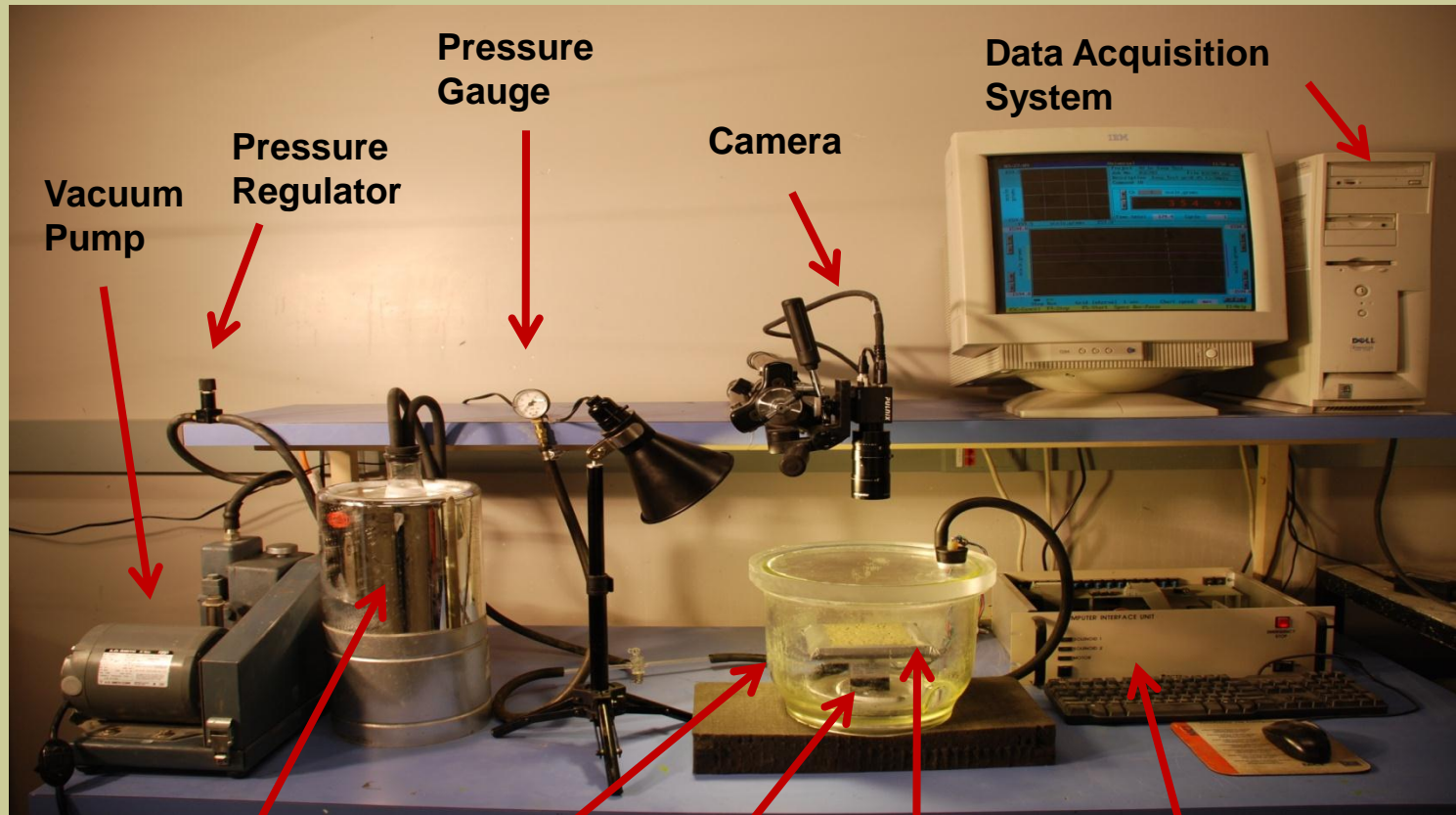
A- HD24

B- HP12

C- Control4

ASU Leakage Cracking Test Setup

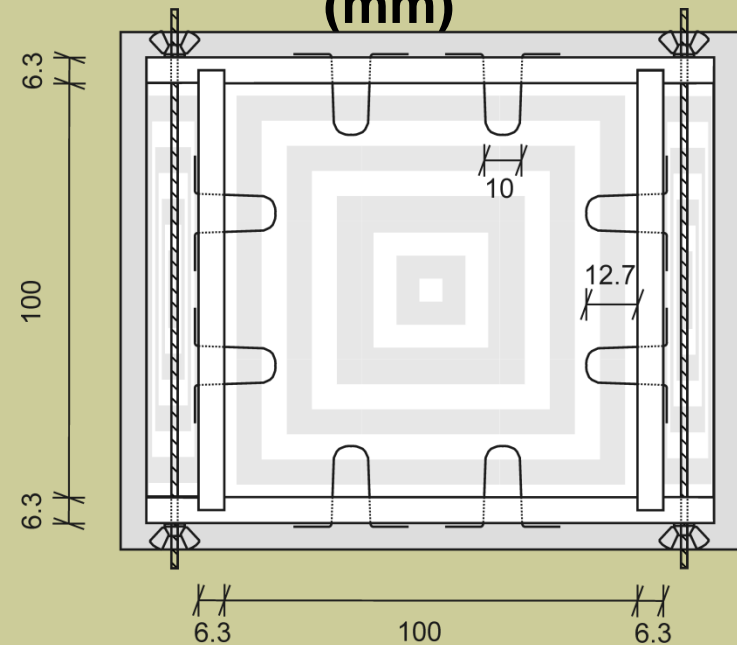
- Evaporation is simulated under low pressure condition.





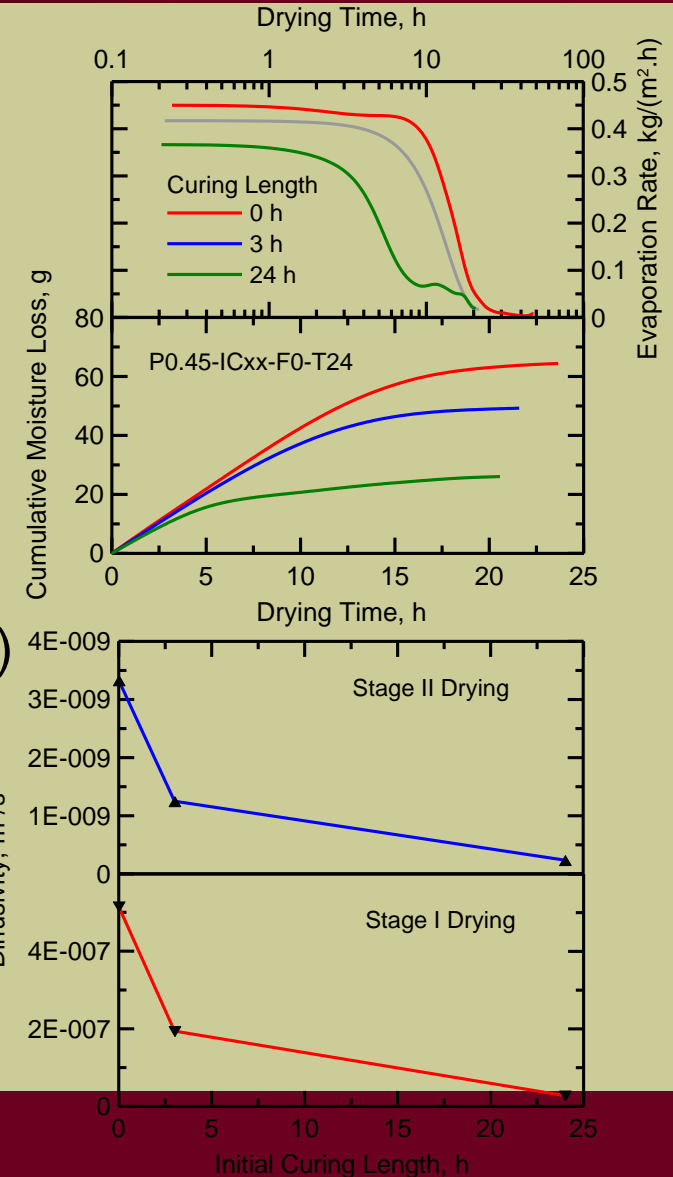
- Load cell serves a scale after calibration to weigh the sample continuously.

Plan view and of the mold
(mm)

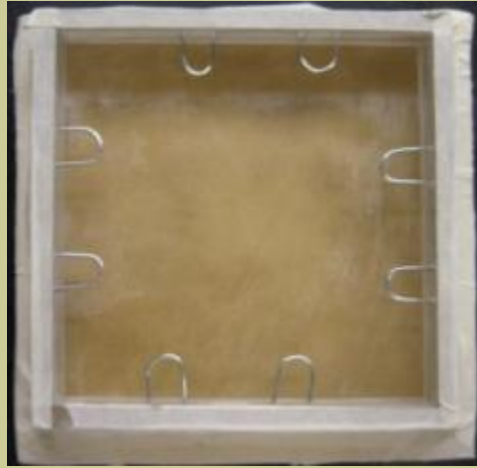


- The mold consists of interlocking pieces made of Polycarbonate
- Anchor hooks are used to fully connect the paste with the mold, providing restraints in two directions

- Cumulative moisture loss:
Decreased from 65 to 49 and 26 g
Translates to 24% and 60% reduction
- Evaporation rate:
 - Initial Evaporation rate:
decreased from 0.42 to 0.39 and 0.34
 - Evaporation rate at 12 h:
decreased from 0.3 to 0.2 and 0.07 kg/(m²h)
- Transition time:
Changed from 9.7 h to 7.3 h and 3 h



■ Moisture diffusivity: →
 D decreased by as much as 62% and 93%



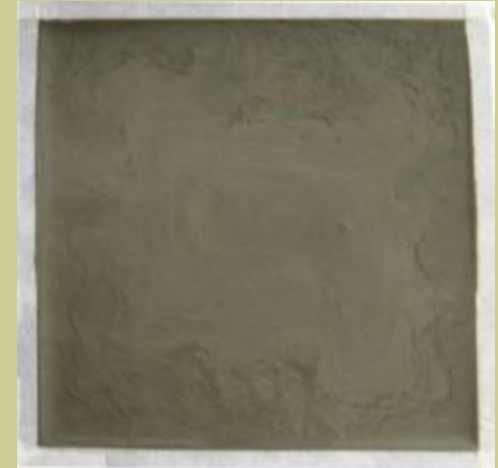
Restrained Molds
(4"× 4"× 0.5")

→
castin
g



Pouring cement paste

→
t =
30 min



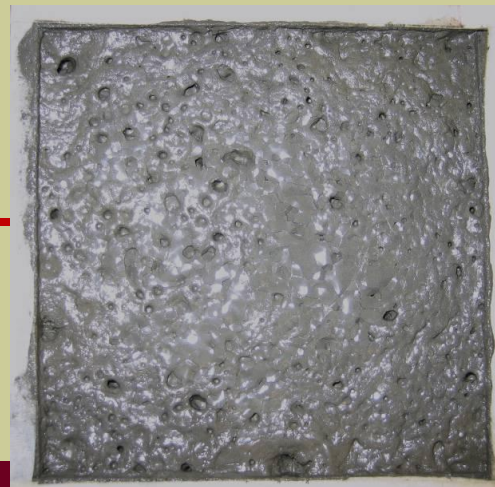
A film of water on
surface

t =
1 hr
↓



cracking on paste surface

t =
4 hr
←



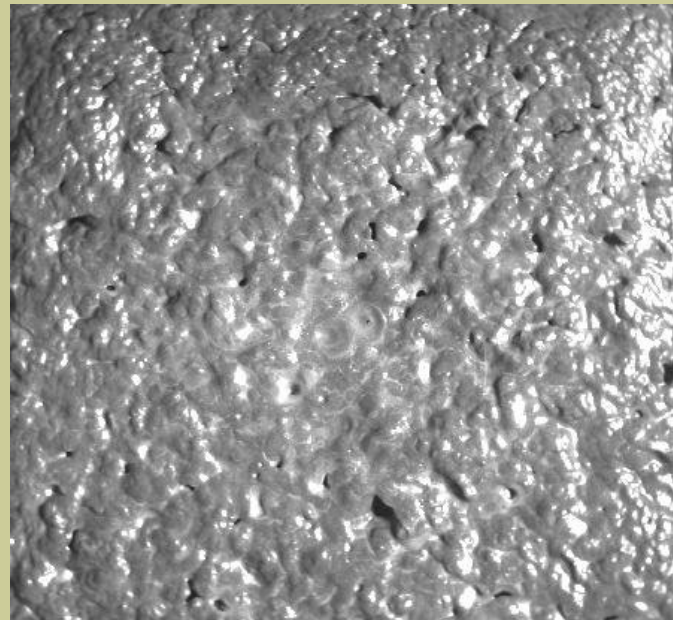
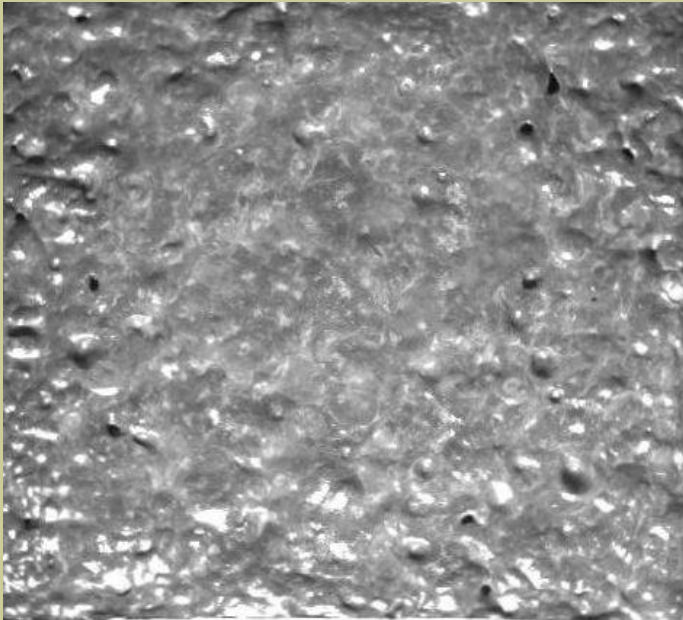
Formation of voids on paste surface

t =
2 hr
←



No layer of water on surface

ASU Time-Lapse Photography (Effect of w/c ratio)



w/c = 0.45

w/c = 0.55

FULTON

school of engineering

w/c = 0.65

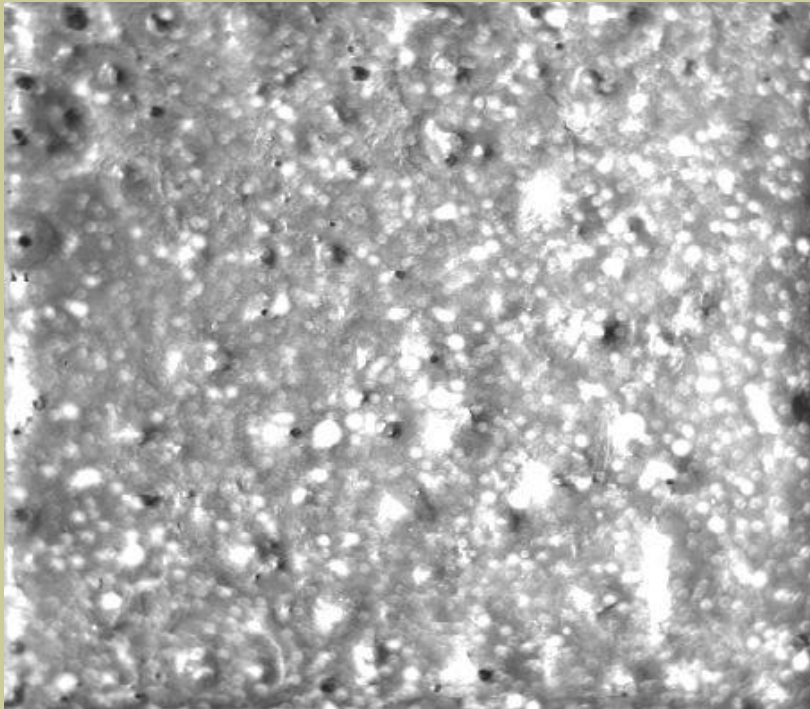
**Drying
Time:**

07:00

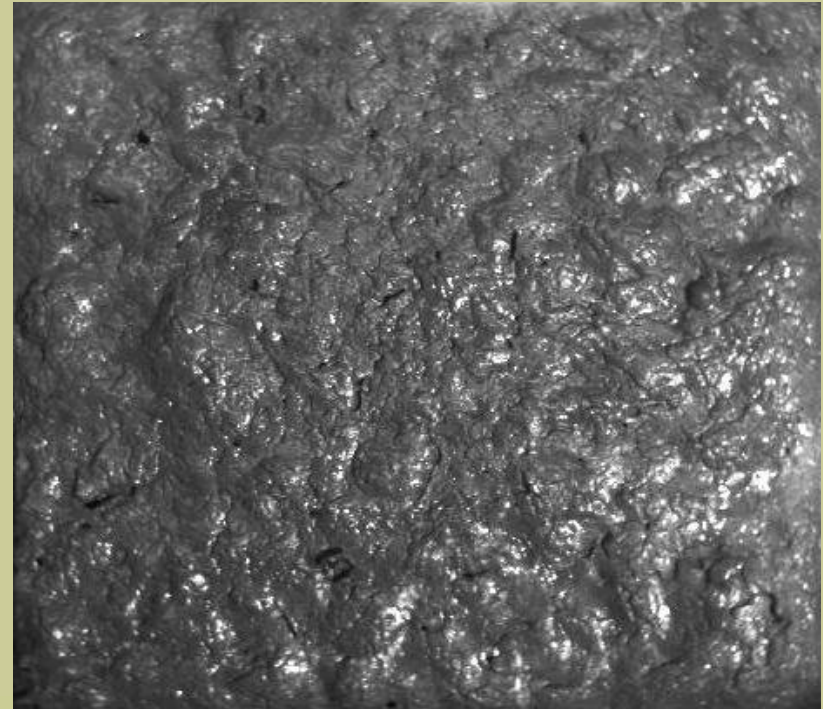


Results of Time-Lapse Photography

Paste



**Fabric Reinforced
Paste**



**Drying
Time:**

FULTON

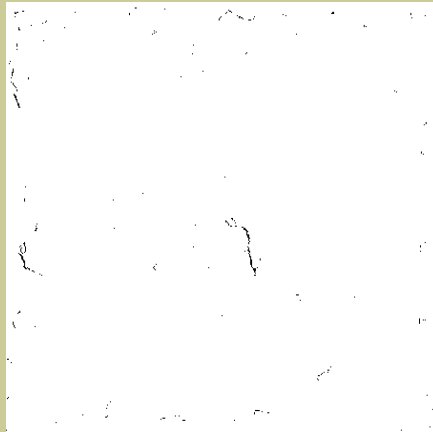
school of engineering

07:00

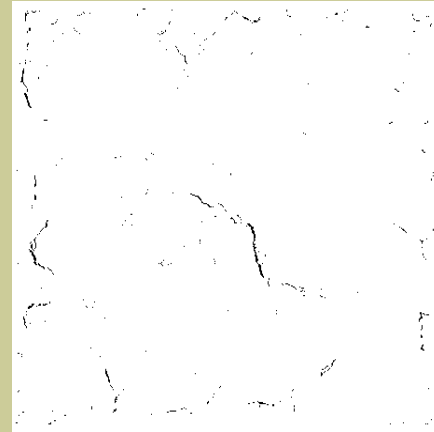
Crack Pattern Evolution



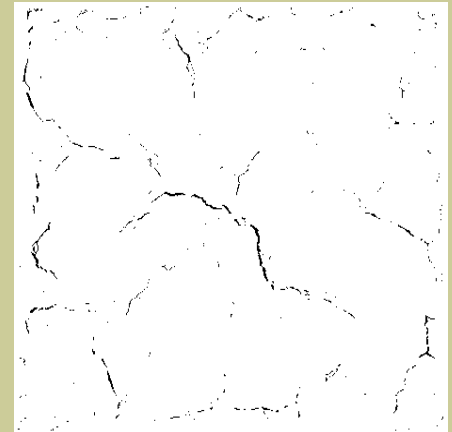
after 3.5 h



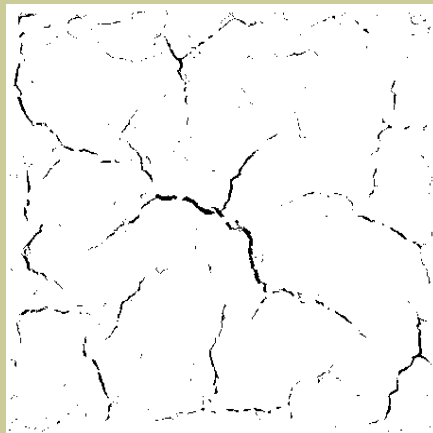
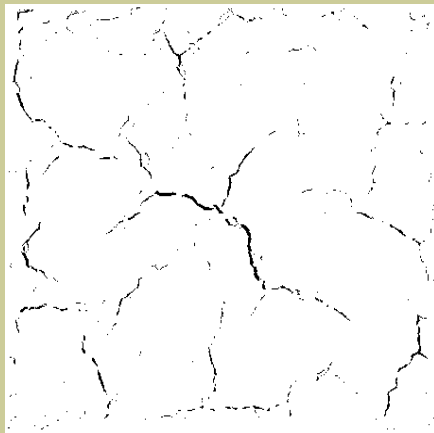
after 3.75 h



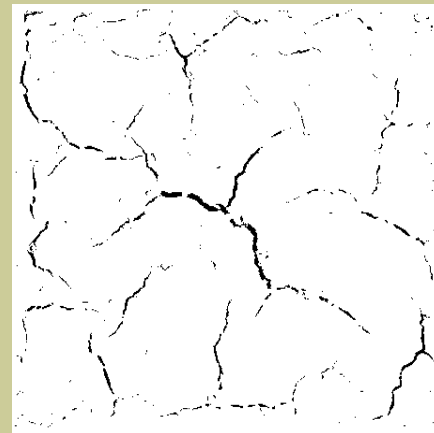
after 4 h



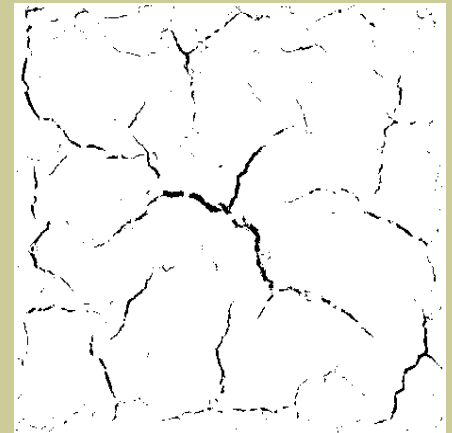
after 4.25 h



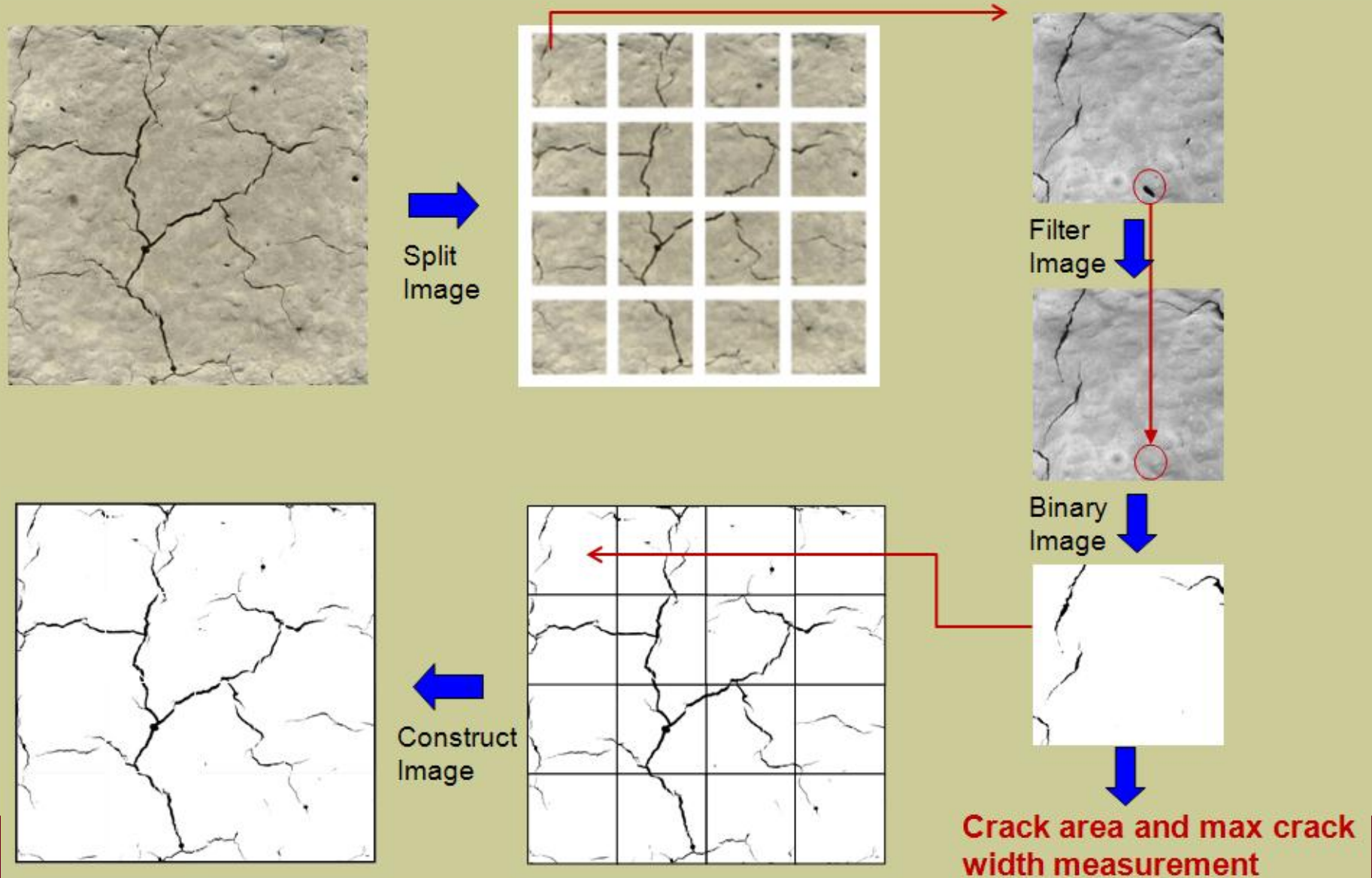
after 4.75 h



after 5 h

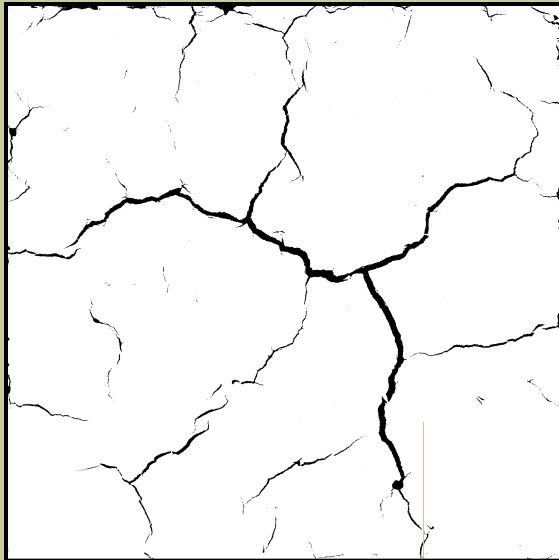


after 4.25 h

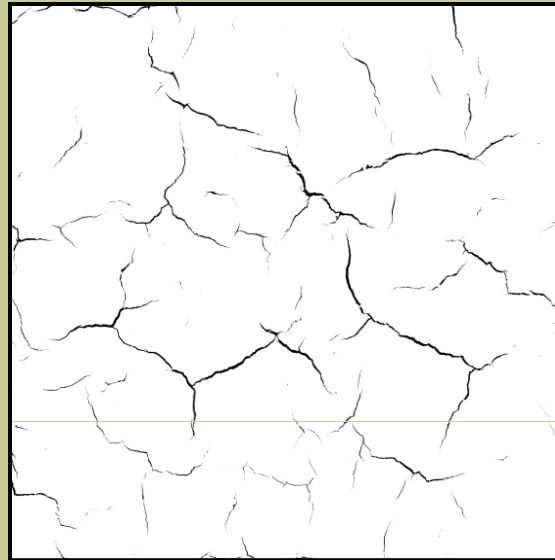


Results of image analysis on 2D cracks

Plain Concrete



Fiber content = 3 kg/m³



Fiber content = 6 kg/m³



Test Series	Max. Crack Width (mm)	Reduction in Max. Crack Width by Adding Fiber (%)	Areal Fraction of Cracks (mm ² /mm ²)	Reduction of Crack Area by Adding Fiber (%)
F0	1.28	-	0.018	-
F3	0.68	47%	0.014	25%
F6	0.37	134%	0.007	84%

- Shrinkage in concrete materials is a major durability concern.
- Adequate Curing, minimizing paste content, using largest aggregate size, reducing water remand, and novel admixtures are some the methods of mitigation of shrinkage cracking.
- Fibers provide internal bridging mechanisms for the closure of the cracks.
- Test methods and modeling techniques can be used to better address the performance of concrete materials



Acknowledgements

- ADOT Materials Group- Paul Sullivan
- ADOT Research Center- Christ Dimitroplos